



NHL-DEM, a Numerically Homogenized Constitutive Law that works in FEM-DEM multi-scale computations

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GRENOBLE INP

UJF GRENOBLE I

Outline

1. Introduction
2. FEM-DEM multiscale modelling : Principle
3. NHL as a constitutive model : ACTIV integration
4. NHL performances (illustrations...)
5. Conclusions & Perspectives



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Multiscale Numerical approach of composite materials : a fast emerging framework

Double-scale approaches : concurrent & collaborating numerical analysis at 2 scales : Macro scale and microscale

- FE² (finite element square)

- FEM X DEM

- ...

A very good review paper (2009) :

Arch Comput Methods Eng (2009) 16: 31–75
DOI 10.1007/s11831-008-9028-8

ORIGINAL PAPER

Multiscale Methods for Composites: A Review

P. Kanouté · D.P. Boso · J.L. Chaboche · B.A. Schrefler



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Multiscale Methods fo

P. Kanouté · D.P. Boso · J.L. Chabo

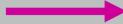
Abstract Various multiscale methods are reviewed in the context of modelling mechanical and thermomechanical responses of composites. They are developed both at the material level and at the structural analysis level, considering sequential or integrated kinds of approaches. More specifically, such schemes like periodic homogenization or mean field approaches are compared and discussed, especially in the context of non linear behaviour. Some recent developments are considered, both in terms of numerical methods (like FE²) and for more analytical approaches based on Transformation Field Analysis, considering both the homogenization and relocalisation steps in the multiscale methodology. Several examples are shown.



Introduction : bridging scales in Geomechanical modelling



**A continuum media
or
an assembly of particles ?**

Continuum : FEM	Particles : DEM
<ul style="list-style-type: none">☺ well suited to Real scale problem☹ CAN NOT realistically model their discrete nature	<ul style="list-style-type: none">☺ Reproduces « naturally » the complex behaviour of grains assembly : cyclic response, anisotropy, strain path dependency☹ Computation time depends on the number of grains -> high CPU costs > limitation to small problems
 Coupling FEM-DEM ☺ ☺	



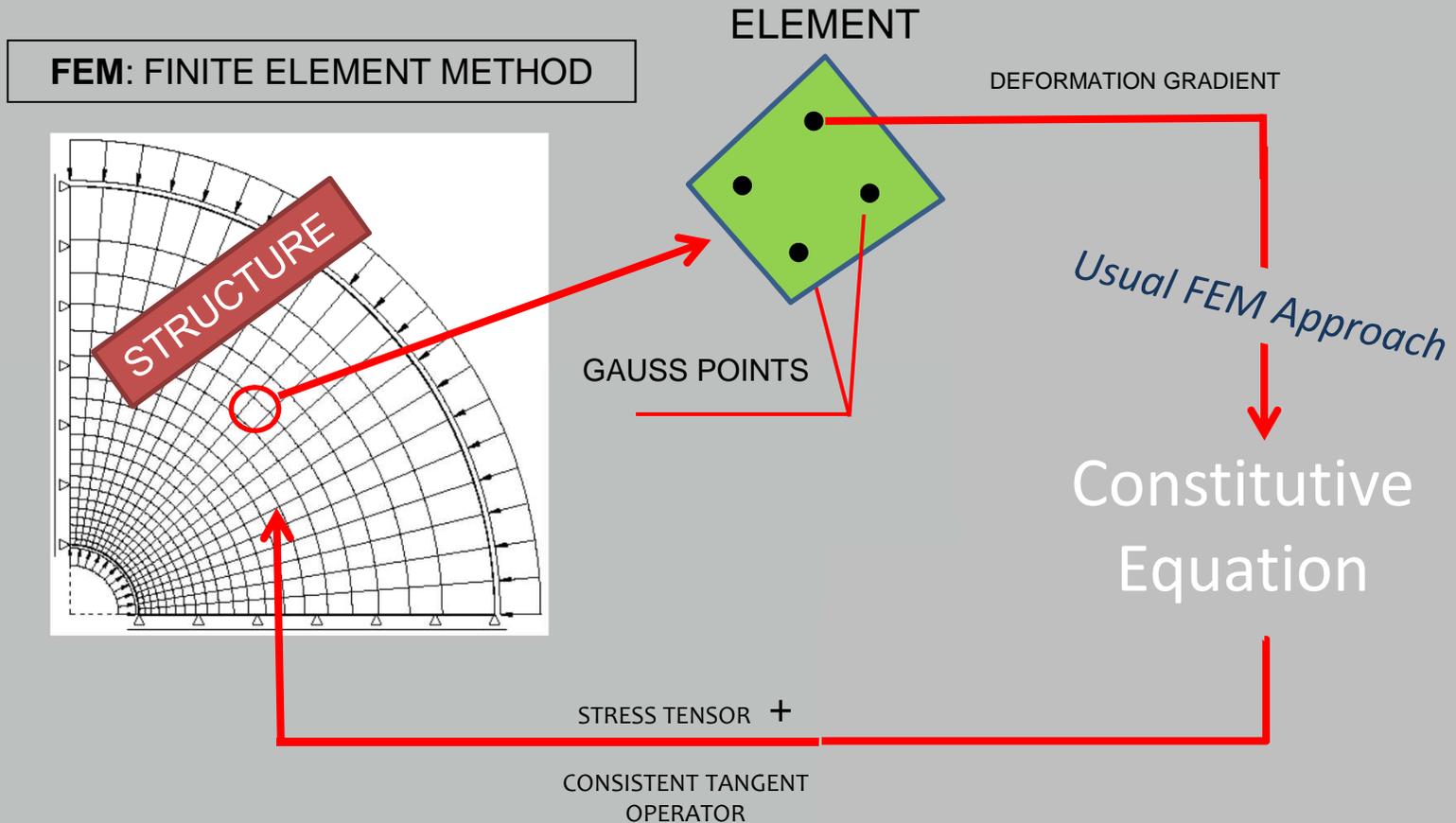
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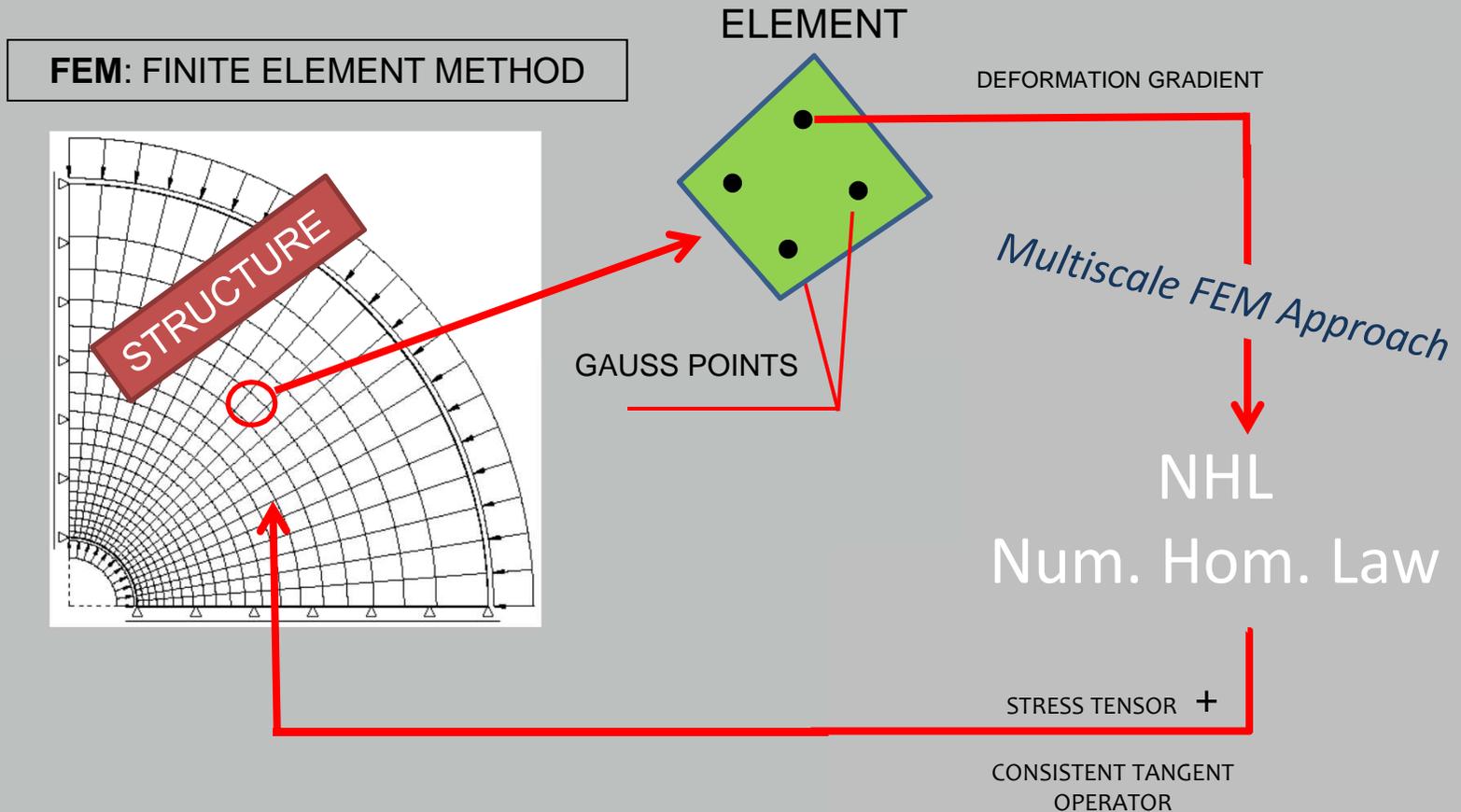
Principle

Introducing a two-scale numerical homogenization approach by FEM - DEM



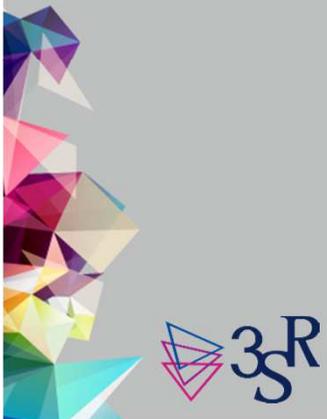
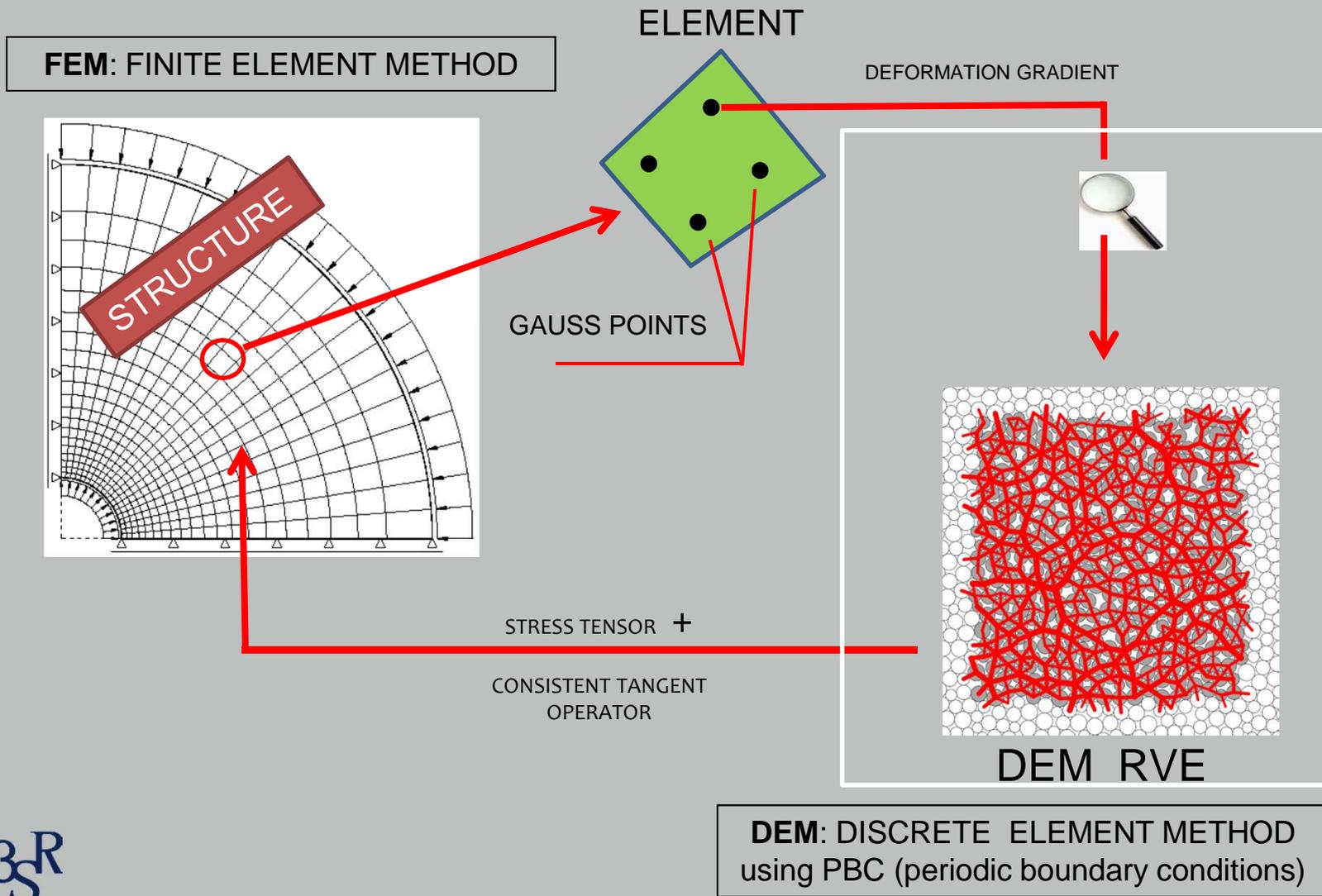
Principle

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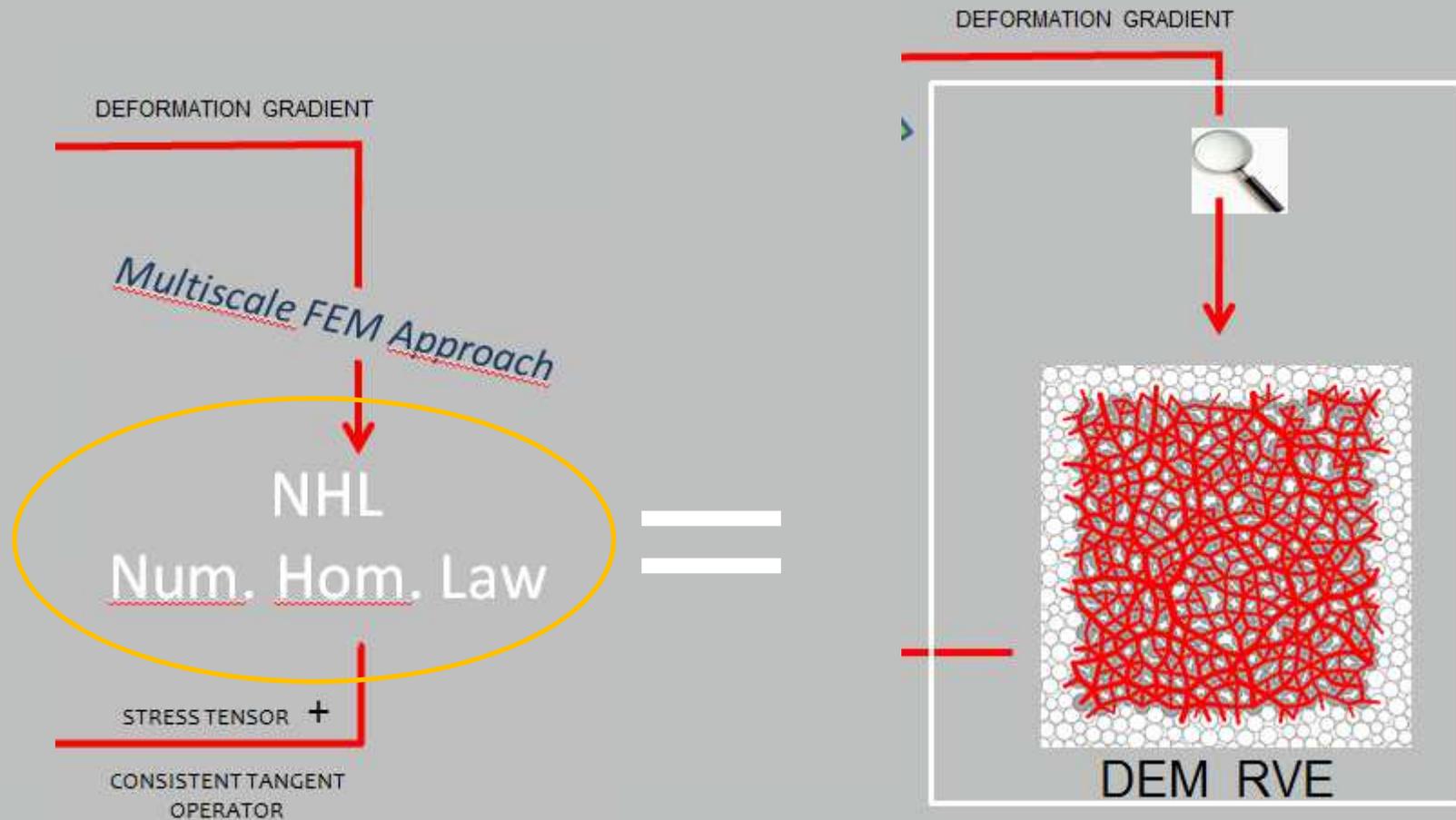


Principle

A two-scale numerical homogenization approach by FEM - DEM

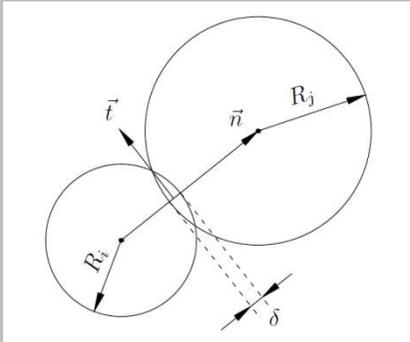


NHL - DEM : DEM-based NHL

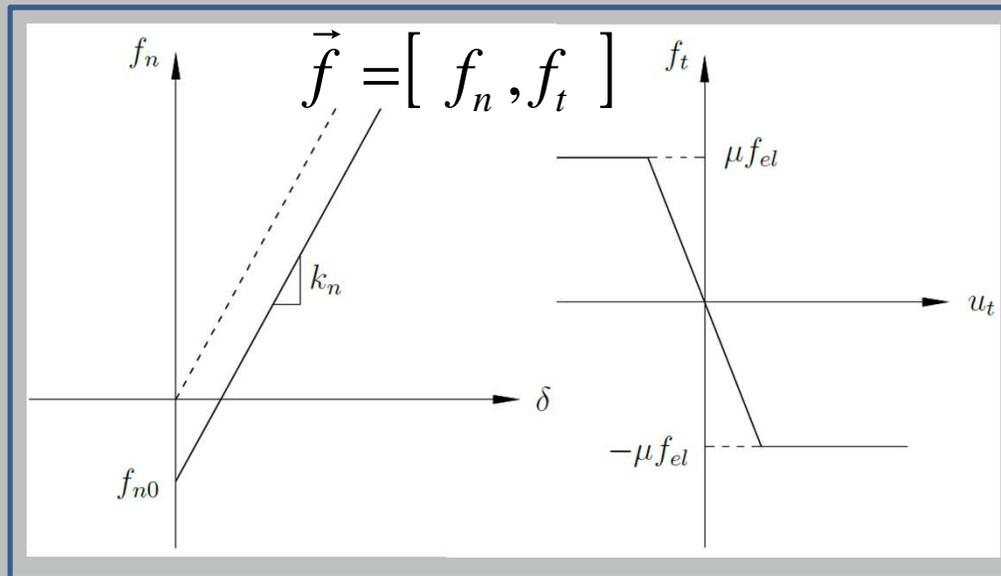


Micro-scale Model

Contact laws *



Discrete Element Method
(Soft contact dynamics type)
with bi-Periodic **B**oundary
Conditions



- Normal repulsive contact force

$$f_{el} = k_n \cdot \delta$$

$$\begin{cases} \delta > 0 & \text{Contact present} \\ \delta = 0 & \text{No contact} \end{cases}$$

- Tangential contact force

$$\delta f_t = k_t \cdot \delta u_t$$

- Coulomb condition

$$\|f_t\| \leq \mu \cdot f_{el}$$

- Cohesion

$$f_n = f_{el} + f_{n0}$$

f_{n0} : cohesive force

$$f_{n0} = p^* \cdot \sigma_0 \quad p^* = 1, 2, \dots$$

Macrosc. Stress tensor :

$$\sigma_{ij} = \frac{1}{S} \cdot \sum_{k=1}^{N_C} f_i^k \cdot l_j^k$$

* : (e.g. Gilibert et al., 2007)



Principle

What do we need ? a **FEM code** + a **DEM code** + a **bridging procedure**

- ▶ **FEM code :**
the choice made has been to use a large multi-purpose FEM code (Lagamine, Liège University Ulg)
- ▶ **DEM code :** an as-compact-as-possible DEM kernel
-> in-house 3SR-Grenoble DEM code, Geochanics team
strong requirement : quasi-perfect static equilibrium at the end of each DEM step
- ▶ **Bridge :**
direct incorporation of the DEM code as a constitutive law in the FEM code (convenient for sequential programming, or OpenMP parallel programming)



Principle

- ▶ We develop this framework since 2008
- ▶ The other team currently developing FEMxDEM in the world is in Hong-Kong (JiDong Shao, HK University) essentially along the same lines as our work (after our communication in IWBDG 9th in Porquerolles)

References :

[1] Miehe C, Dettmar J (2004) *Comput. Methods Appl. Mech. Engrg*, 225-256.

[2] Meier HA, Steinmann P, Kuhl E, *Technische Mechanik*, Band 28, Heft 1, 2008, 32-42.

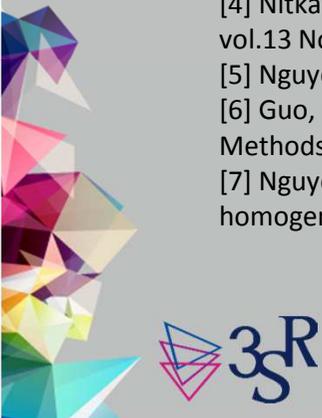
[3] Kouznetsova V, Brekelmans WAM, Baaijens FPT (2001) *Computational Mechanics* 27 37-48.

[4] Nitka M., Combe G., Dascalu C., Desrues J. (2011) Two-scale modeling of granular materials: a DEM-FEM approach, *Granular Matter* vol.13 No 3, pp. 277-281

[5] Nguyen TK, Combe G, Caillerie D, Desrues J (2013) *AIP Conf, Proc.* 1542, 1194

[6] Guo, N Zhao, JD (2014) A coupled FEM/DEM approach for hierarchical multiscale modelling of granular media, *Int. J. for Numerical Methods in Engineering*, Vol.99, No 11, Pages: 789-818

[7] Nguyen, Trung Kien; Combe, Gael; Caillerie, Denis; Desrues, J. (2014) FEM x DEM modelling of cohesive granular materials: Numerical homogenisation and multi-scale simulations, *ACTA GEOPHYSICA* V.62 No 5 pp: 1109-1126



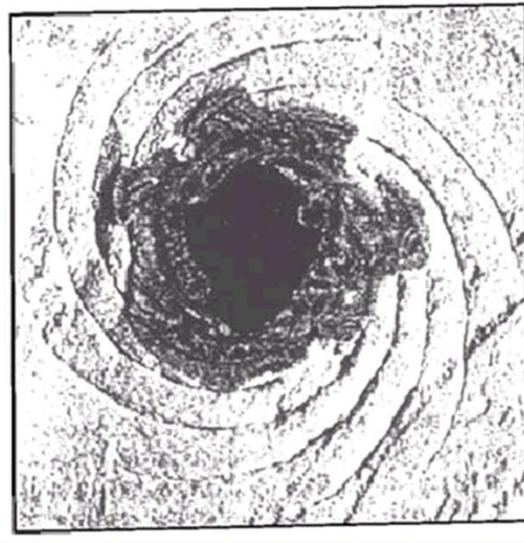
But first of all, does it really work ?

- ▶ Too complex ...?
- ▶ Too CPU demanding ...?
- ▶ only of academic interest ... ?

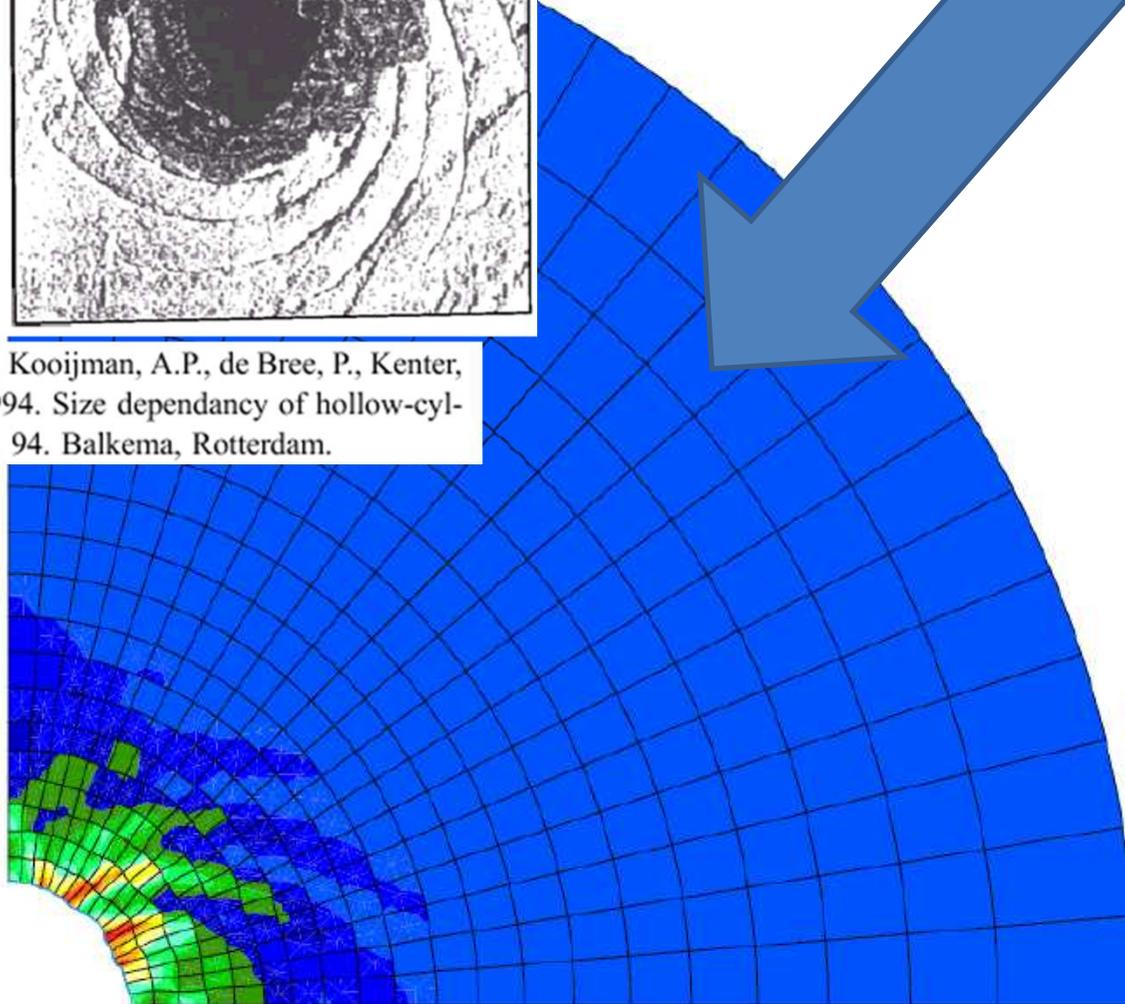
- ▶ Just a few examples



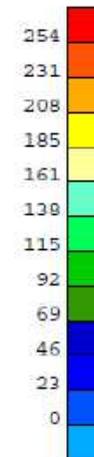
Multiscale FEM-DEM Computations

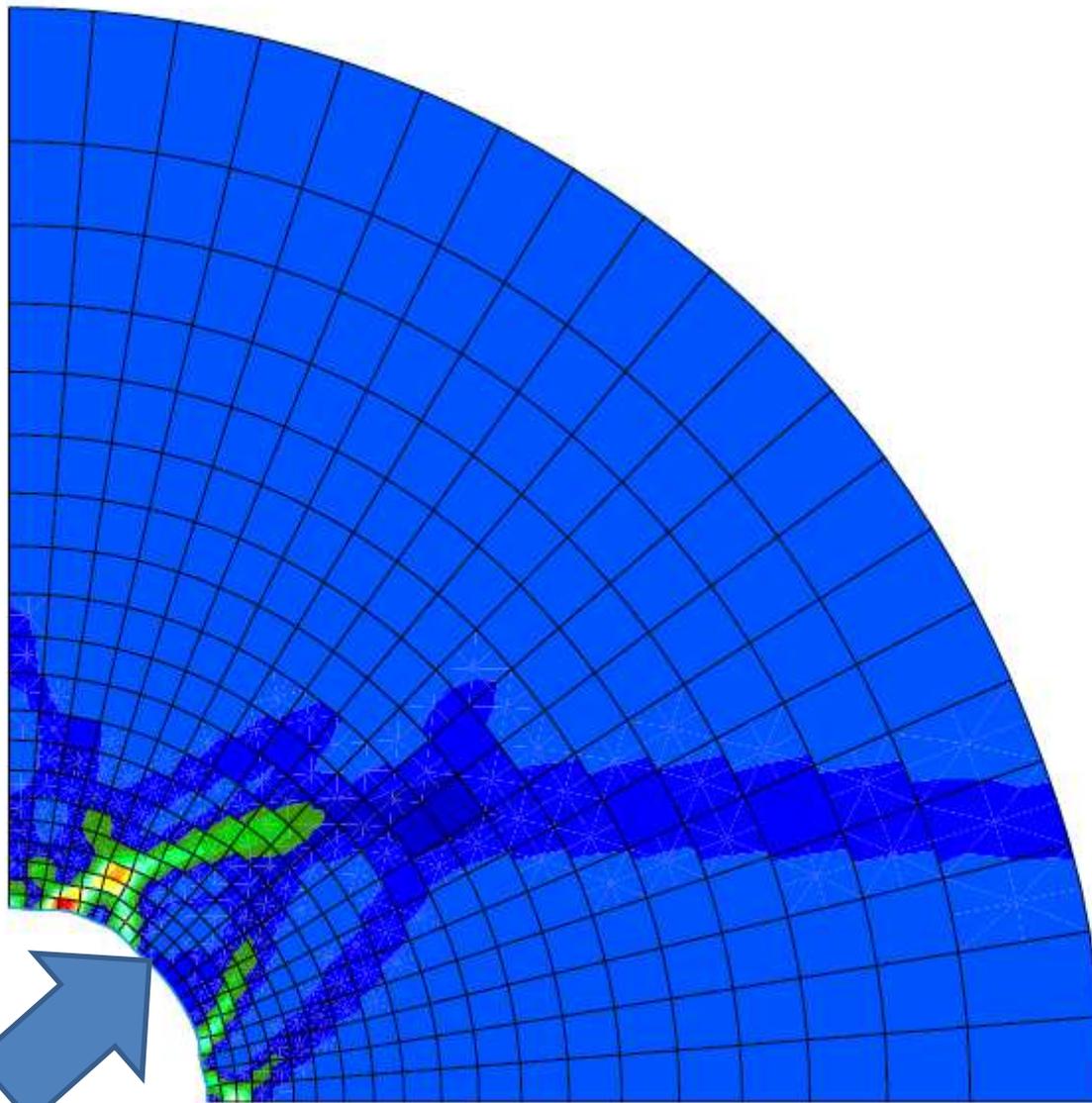


van den Hoek, P.J., Smit, D.-J., Kooijman, A.P., de Bree, P., Kenter, C.J., Khodaverdian, M., 1994. Size dependency of hollow-cylinder stability. Eurock, vol. 94. Balkema, Rotterdam.



* 1.000E-03





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DELT= 0.305E-01
 X 0.100E+04
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* 1.000E-03

	MIN	MAX
X	0.000	4.360
Y	0.000	4.325
Z	0.000	0.000

SELECTION DES ELEMENTS
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1/4 cylindre creux Q8 - maillage progressive p1

tknguyen

Cylindre_creux_Q8

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NHL-DEM as a constitutive model

- ▶ NHL *is* a constitutive model in the general form : $\mathbf{F}_{\tau \leq t} \rightarrow \boldsymbol{\sigma}_{\tau \leq t}$
with $\mathbf{a}_{\tau \leq t}$ history of the function \mathbf{a} i.e. $\mathbf{a}_{\tau \leq t} = \{\mathbf{a}(\tau), \tau \in [0, t]\}$

Building the constitutive law $\mathbf{F}_{\tau \leq t} \rightarrow \boldsymbol{\sigma}_{\tau \leq t}$ is an evolution problem

- ▶ NHL is built using a **step by step** method which rely on the numerical homogenisation of a RVE modelled using DEM : $\mathbf{F}^n \rightarrow \boldsymbol{\sigma}^n$

If \mathbf{F}^n and $\boldsymbol{\sigma}^n$ are the values of \mathbf{F} and $\boldsymbol{\sigma}$ at the end of the step n , then we write :

$$\boldsymbol{\sigma}^n = \boldsymbol{\Sigma}^n (\mathbf{F}^n)$$

assuming that $\boldsymbol{\Sigma}^n$ is differentiable we write :

$$\delta \boldsymbol{\sigma}^n = \boldsymbol{\Sigma}^n (\mathbf{F}^n + \delta \mathbf{F}^n) - \boldsymbol{\Sigma}^n (\mathbf{F}^n) = \mathbf{C}^n : \delta \mathbf{F}^n + \dots$$

with \mathbf{C}^n a four rank tensor : $\mathbf{C}^n = \frac{d\boldsymbol{\Sigma}^n}{d\mathbf{F}^n}$



NHL-DEM as a constitutive model - I/II

- ▶ The numerical homogenisation approach to constitutive modelling offers **specific and remarkable performances** with respect to **difficult-to-model material behaviours** :
- ▶ / strain-softening / inherent and induced anisotropy / principal stress rotation / cyclic response / compression-extension cycles / ...
- ▶ All these performances results simply from the fact that the **state** of the material (i.e. the RVE) is **exhaustively** described by the set of (state) variables which are :

the position of the grains, the actual list of contacts AND the forces at these contacts
- ▶ Next : ACTIV



NHL-DEM as a constitutive model - II/II

- ▶ To perform the exploration of such constitutive responses, a *material-point* constitutive integration code “ACTIV” is used,
- ▶ ACTIV is an *in-house* 3SR-Grenoble development, initially created with the hypoplastic CLoE model in the years 1990’ (both still in use).
- ▶ ACTIV allows to impose *any combination of stress and strain components* to an elementary volume and compute the constitutive response to this loading program i.e. the *complementary stress and strain components*

Examples :

*Triaxial test : starting from an initial stress state (consistent with the RVE intergranular forces),
i) isotropic loading to a given stress state, then ii) deviatoric loading, either strain-controlled or stress controlled.*

Cyclic triaxial test, either stress or strain controlled



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NHL-DEM performances :

- ▶ Strain softening and strain localisation
- ▶ Cyclic response
- ▶ Principal stress rotations
- ▶ Anisotropy

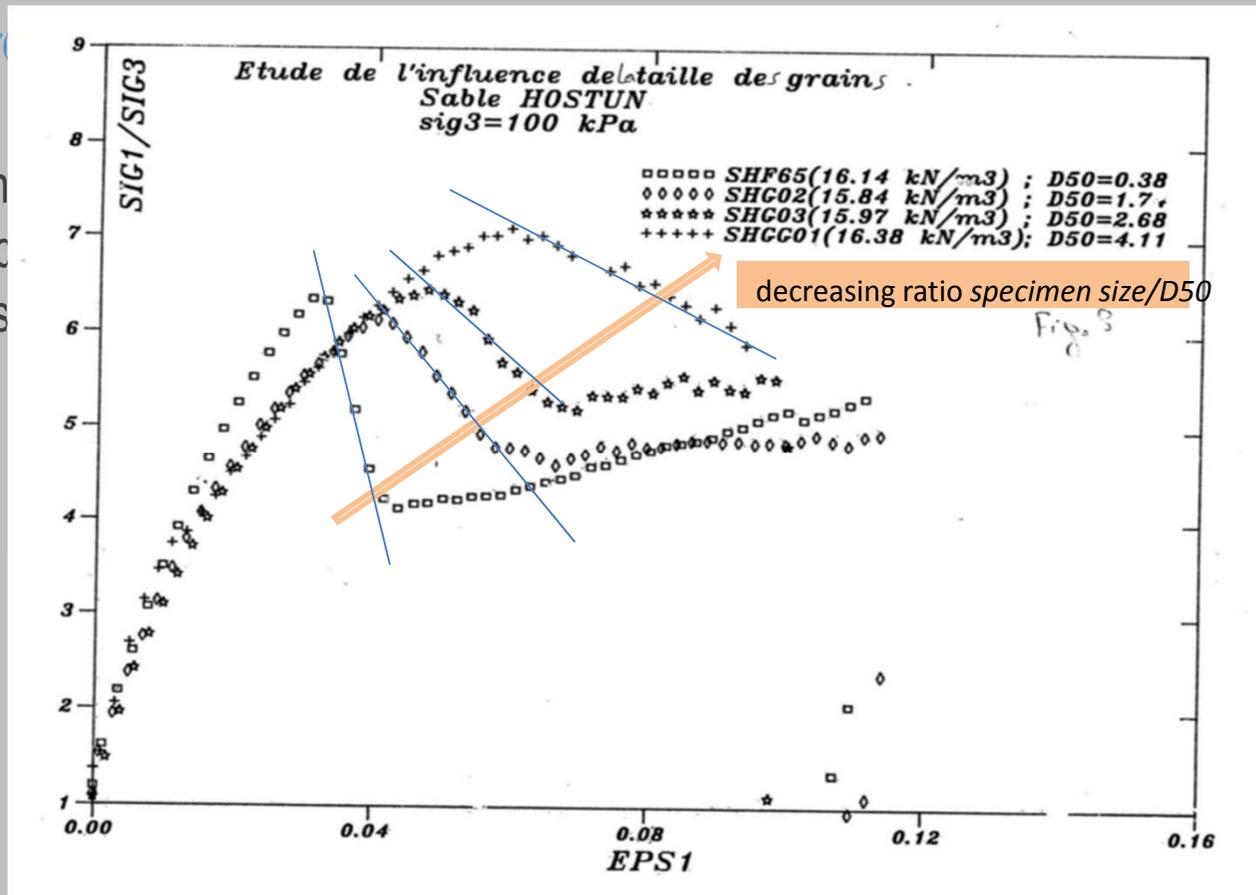


NHL-DEM performances : strain softening and strain localisation

- ▶ Strength reduction of specimens in laboratory tests : a structural response or a material response ? Extremely difficult to assess experimentally
 - ▶ e.g., scale dependency in the test response : due to localisation, smaller specimens show smaller strain softening rate

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- ▶ Then, introducing strain softening in phenomenological constitutive laws
is lacking a sound experimental basis
- ▶ DEM computations provide strain softening on a micro-structural basis : evolution of contacts distribution and orientation, grain rotation, changes in grain distribution (void ratio), destruction of cohesive links



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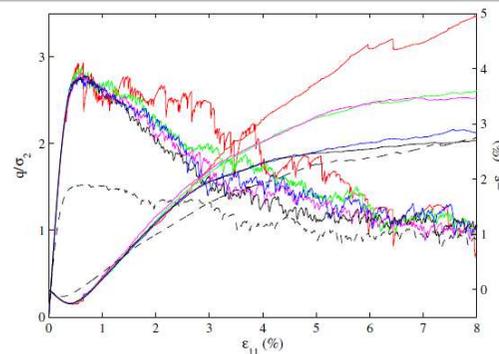
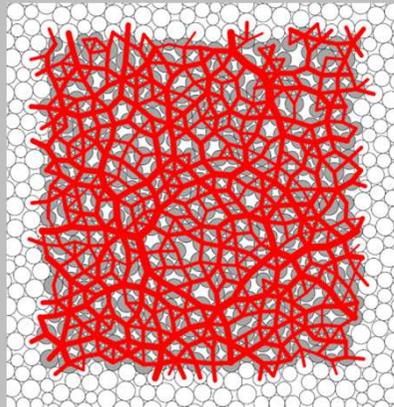


FIGURE 2.18 – Réponses macroscopiques des essais biaxiaux en compression : VER400 (rouge), VER3600 (vert), VER6400 (rose), VER10000 (bleu) et VER22500 (noir). La réponse en noir pointillé est celle du cas sans cohésion du VER10000, avec : $\kappa = k_n / (\sigma_0 \cdot \bar{a}) = 1000$, $k_n / k_t = 1$, $\mu = 0.5$ et $p^* = 0$.

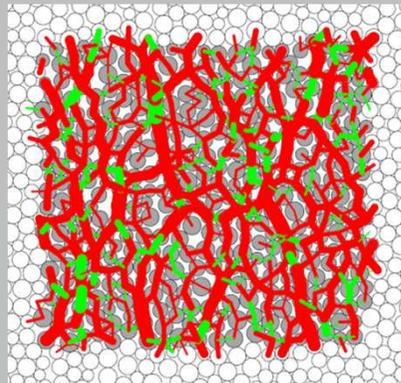


Strain softening : Micro-scale Model response

Biaxial test (DEM with PBC): REV contains 400 particles



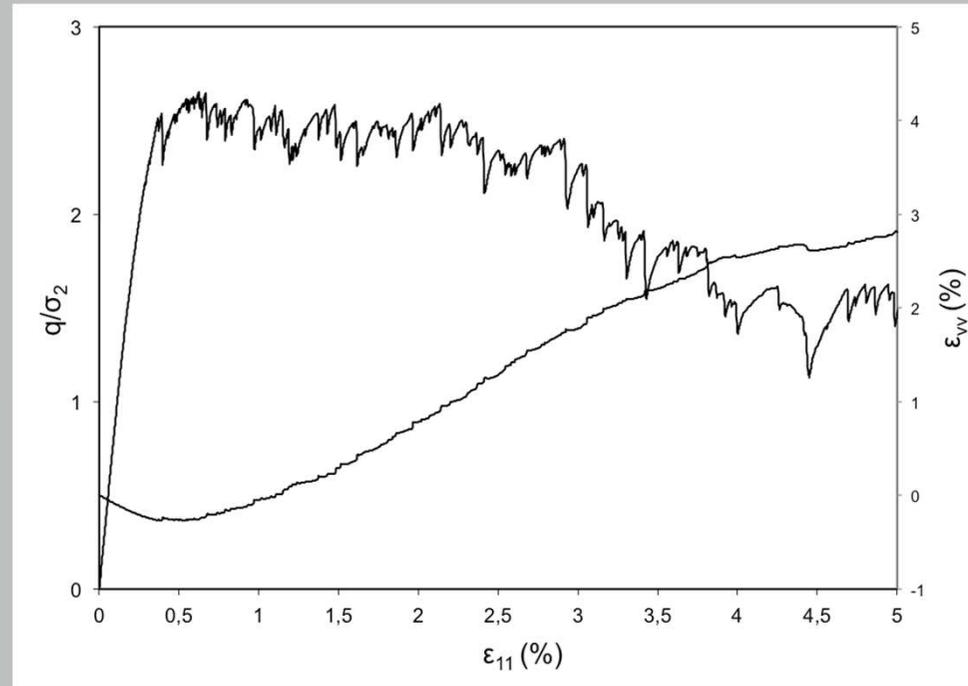
Initial configuration



at 3% of axial strain (ϵ_{11})

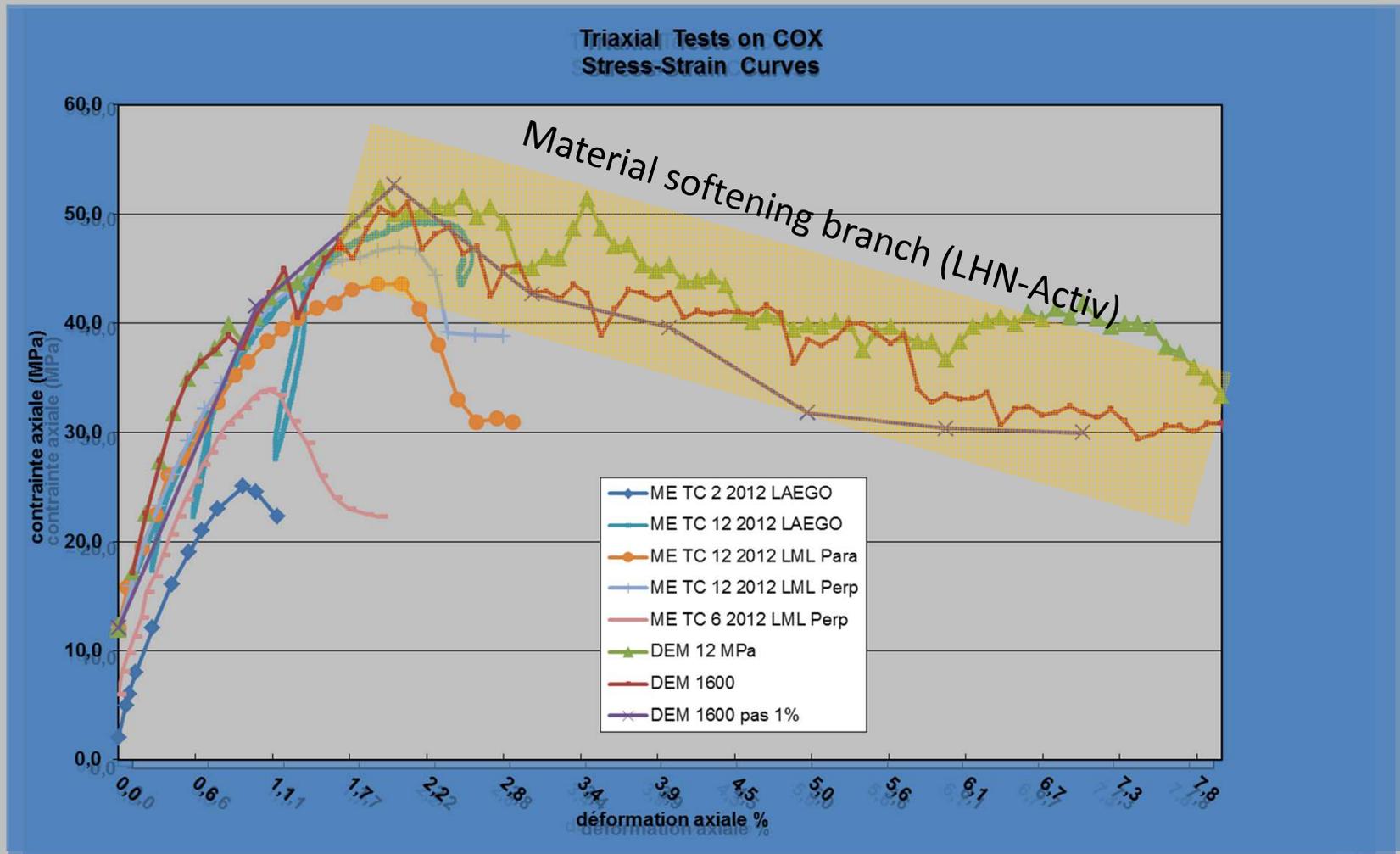
— f_c effective

— $f_c = 0$

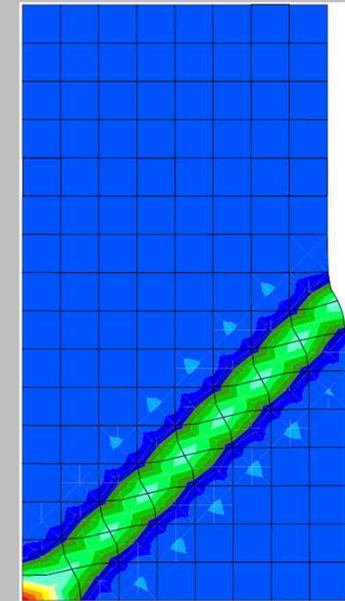
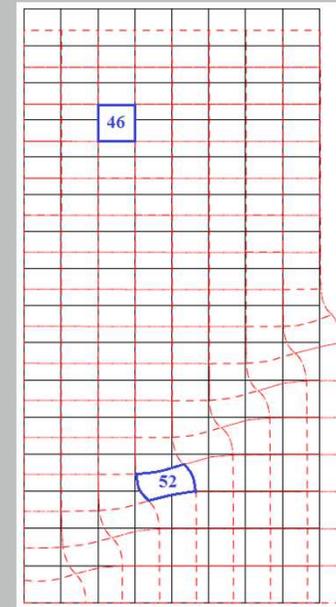
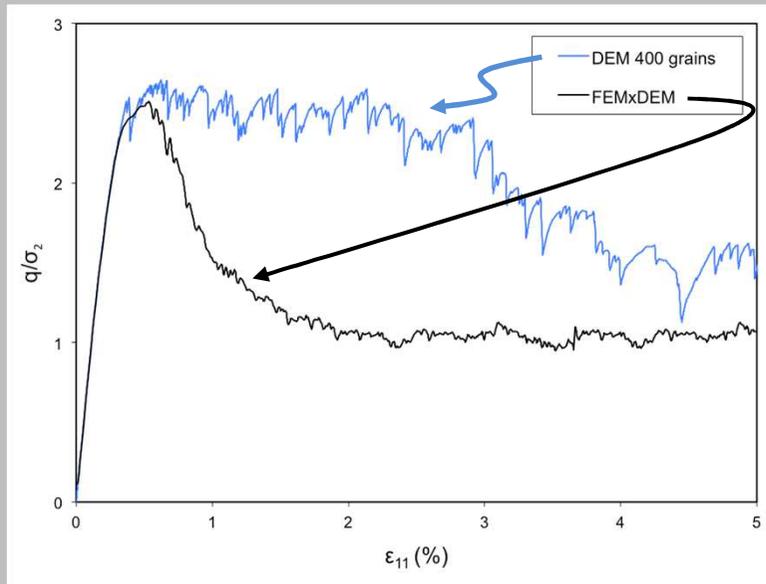


Strain Softening : LHN response (via Activ)

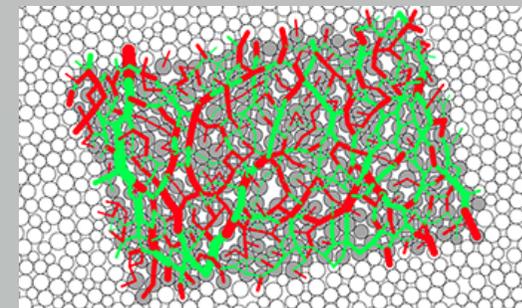
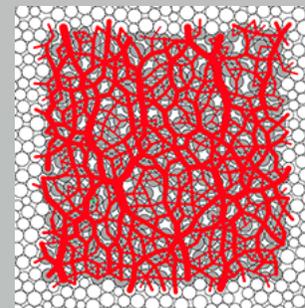
Triaxial test on a cohesive geomaterial



Strain Softening and Strain localization : FEM x DEM response



Deformed structure and second invariant of strain tensor



Element 46

Element 52

Deformed REV

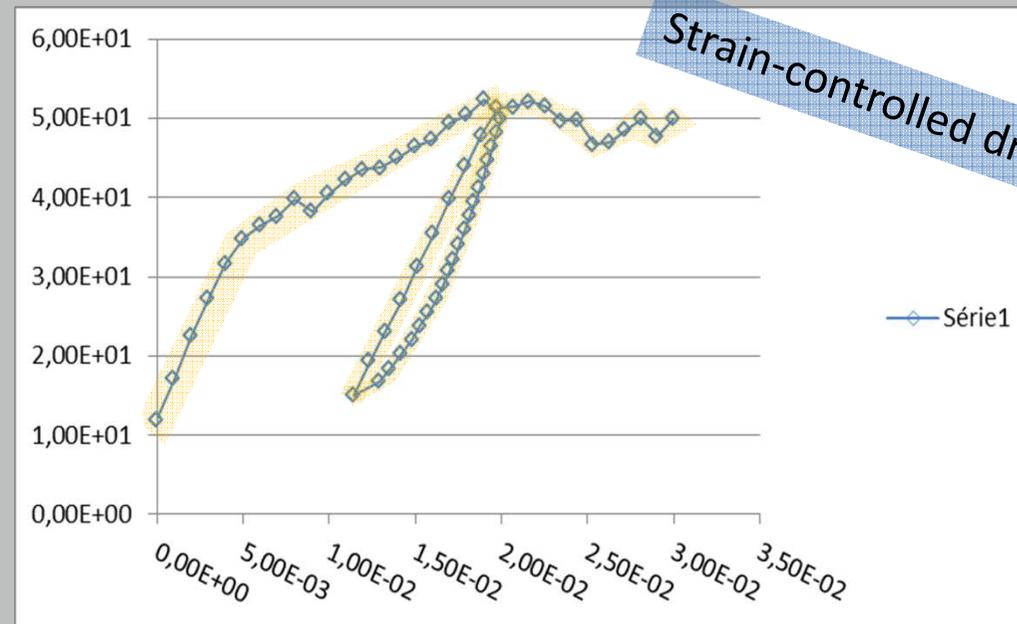


NHL-DEM performances :

- ▶ Strain softening and strain localisation
- ▶ **Cyclic response**
- ▶ Anisotropy
- ▶ Principal stress rotations



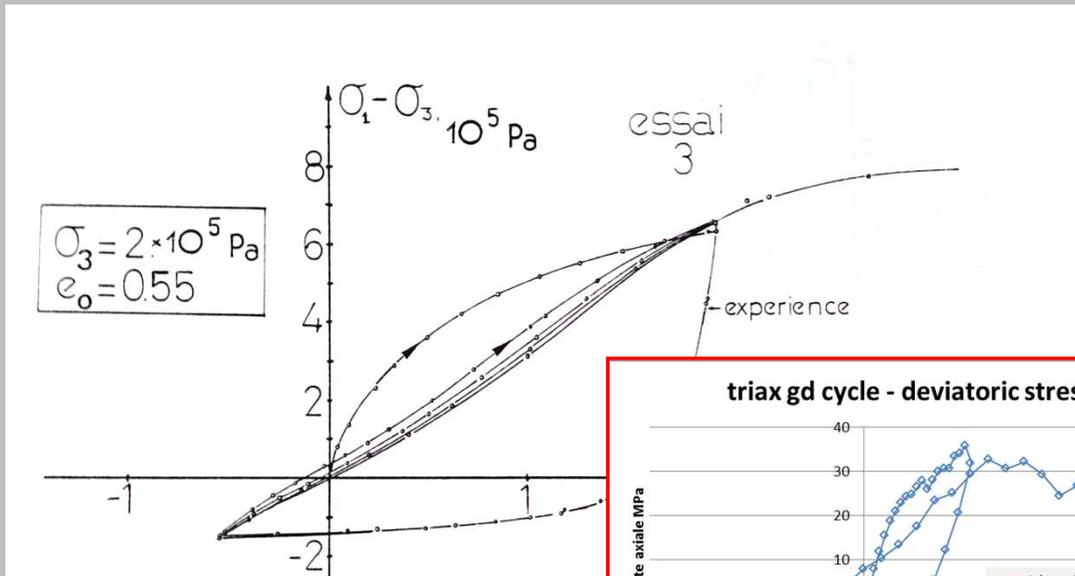
NHL-DEM performances : cycles



- ▶ Simple loading-unloading-reloading :
- ▶ The RVE state variables (grain's position, contacts and contact forces) retain all the information necessary to predict :
- ▶ progressive stiffness degradation upon continuous loading,
- ▶ then quasi-but-not-totally elastic unloading,
- ▶ then elastic reloading
- ▶ up to re-entering the plastic regime



NHL-DEM performances : compression-extension cycles



I. Thanopoulos (1981) "Contributions au comportement cyclique des milieux granulaires" Thèse de Docteur-Ingénieur, Grenoble

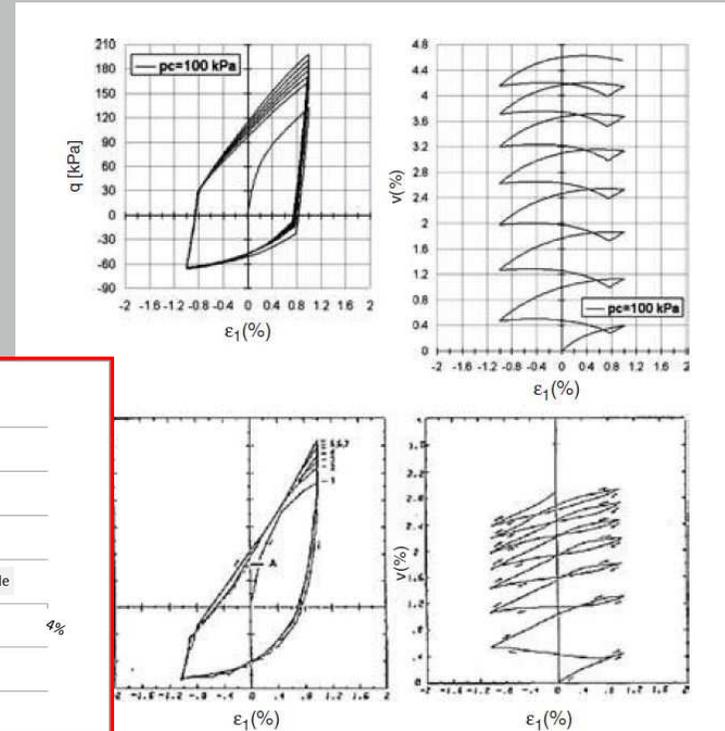
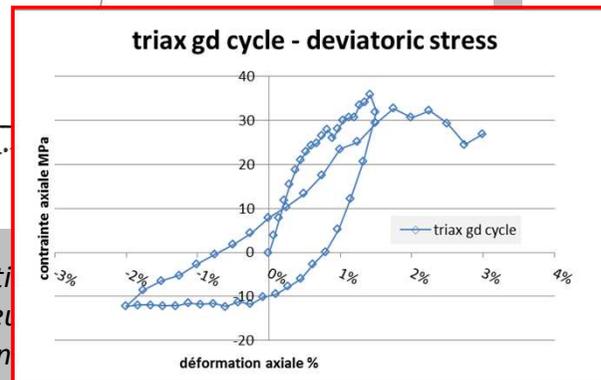


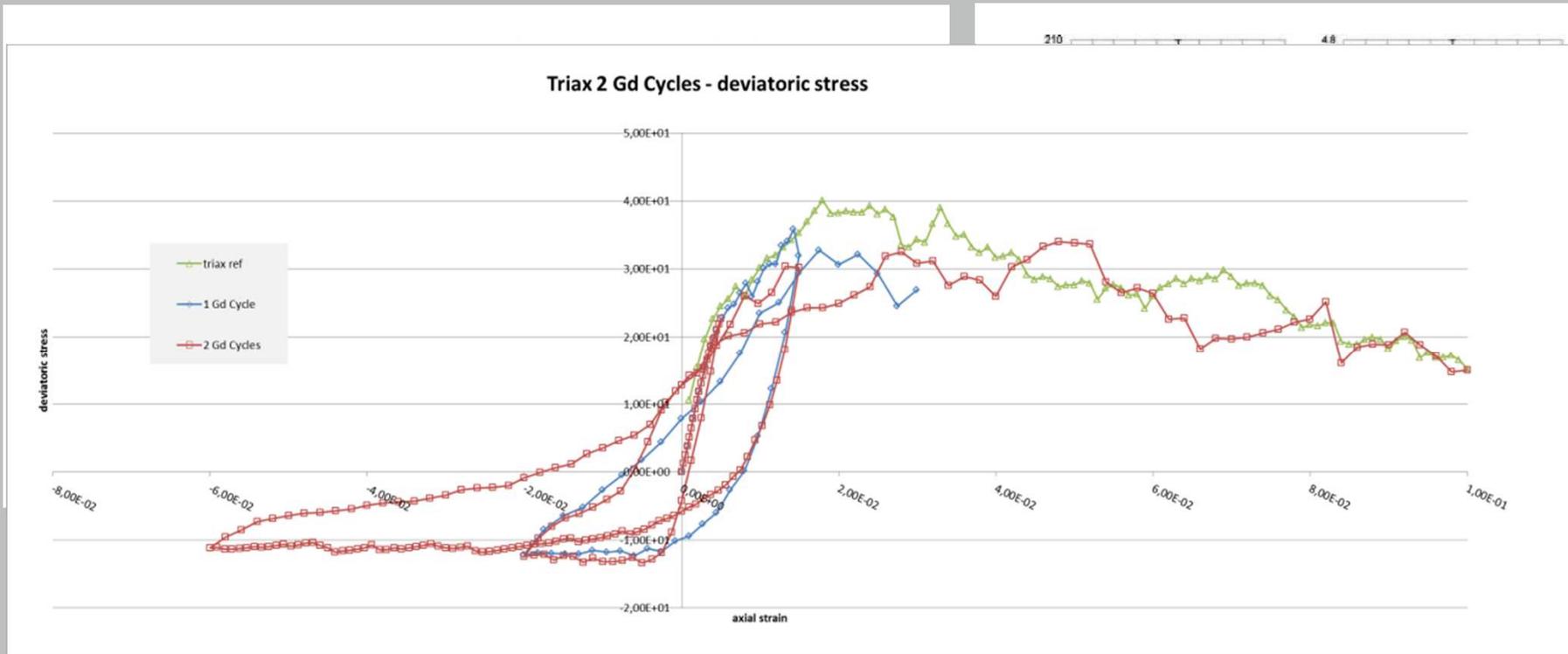
Figure 4. Drained cyclic compression/extension test on medium loose Hostun RF sand: (a) numerical simulations, (b) experimental data (Mohkam 1983).

- ▶ Large compression-extension cycles :
Not impossible to model with formal CE,
...still not easy
- ▶ NHL-DEM provide **without any special development**
a reasonably good response

C. Zambelli, C. di Prisco & S. Imposimato (2004) **A cyclic elasto-viscoplastic constitutive model: theoretical discussion and validation**, in *Cyclic Behaviour of Soils and Liquefaction Phenomena*, Triantafyllidis (ed) © 2004 Taylor & Francis Group, London



NHL-DEM performances : triaxial compression-extension cycles



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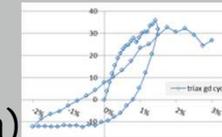
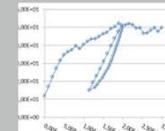
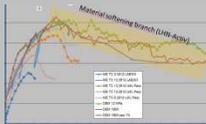
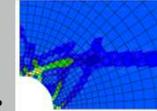
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Conclusions & Perspectives

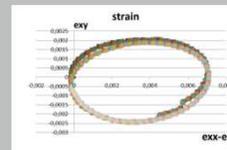
CONCLUSIONS

- We have presented a Two-scale numerical approach for granular materials: combining FEM (at macro scale) and DEM (at micro scale).
- Illustration by 2 examples of BVP on a hollow cylinder analogous to i) underground excavations and drilling, or ii) cavity expansion.
- Focusing on the constitutive law NHL-DEM = numerically homogenised law based on the micro-scale DEM simulation, we have studied the response of the law to simple and complex stress-strain paths :
- Strain softening on triaxial path -> localisation in FEM
- Cyclic response (one cycle, small)
- Cyclic response (one or two large cycles, with compression-extension)
- Multi cyclic response with temporary loss of control



NOT PRESENTED

principal stress rotation
anisotropy



PERSPECTIVES

- Second gradient regularisation
- Parallelisation (massive preferably)
- 3D approach



Thank you
for your attention





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