

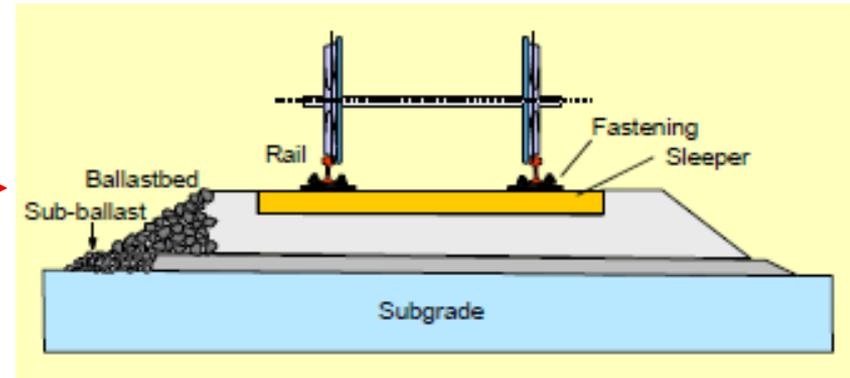


25th ALERT Workshop

Reinforcement of railway ballast using random fibre inclusions

Olufemi Ajayi

Louis Le Pen, Antonis Zervos & William Powrie



Ballast in the sub-structure:

- Reduces bearing pressure on underlying foundation soil
- Retains track alignment



Track settlement



Tamping:

- To restore line and level
- Performed periodically
- Causes ballast breakage
- Faster and more frequent trains
- Railway's working day is becoming longer
- Renewals and maintenance cost £



Geogrids:

- To reduce permanent vertical settlement and lateral spread
- Limitations on tamping

Track materials:

- More resilient
- Cost effective

$D_{50} = 1.8 \text{ mm}$



Fibre reinforced LB sand

e.g. Michalowski & Zhao (1996); Diambra et al. (2010); Dos Santos et al. (2010)

Fibres

$D_{50} = 42 \text{ mm}$

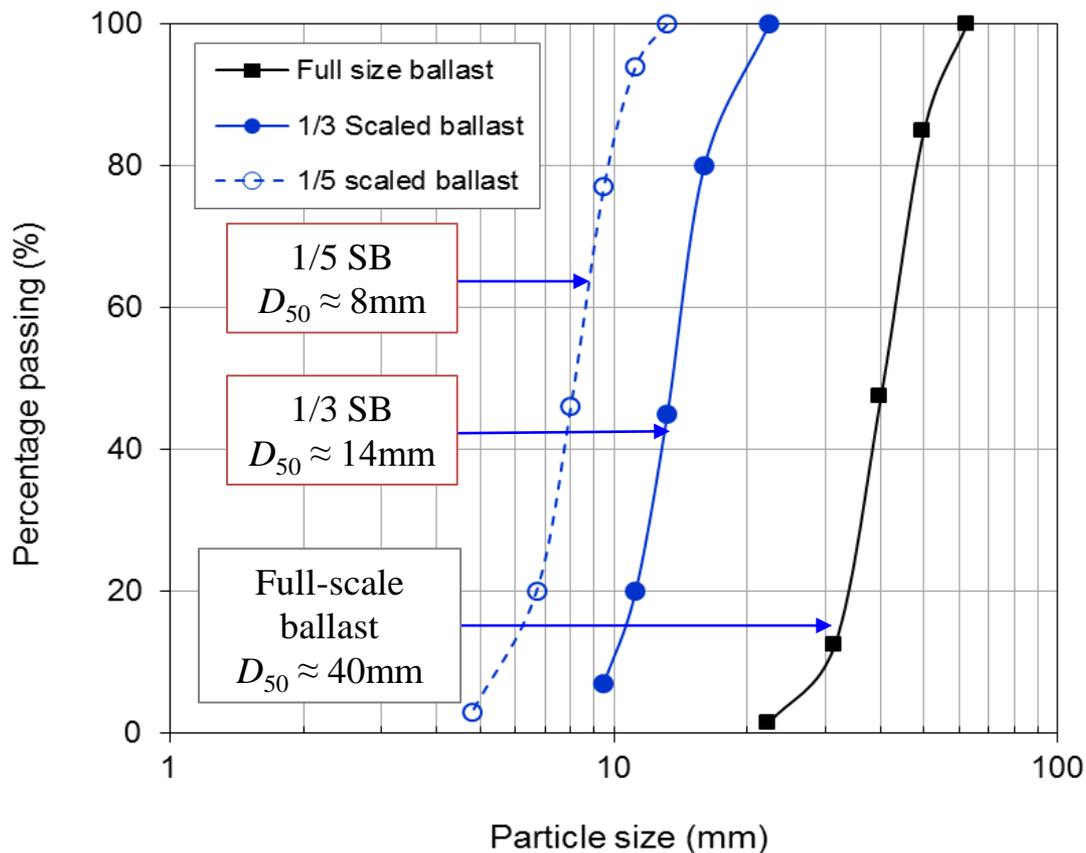


Fibre reinforced ballast

- Influence of fibres on packing of larger aggregates
- Scaling relationships
- Mechanical behaviour in railway ballast

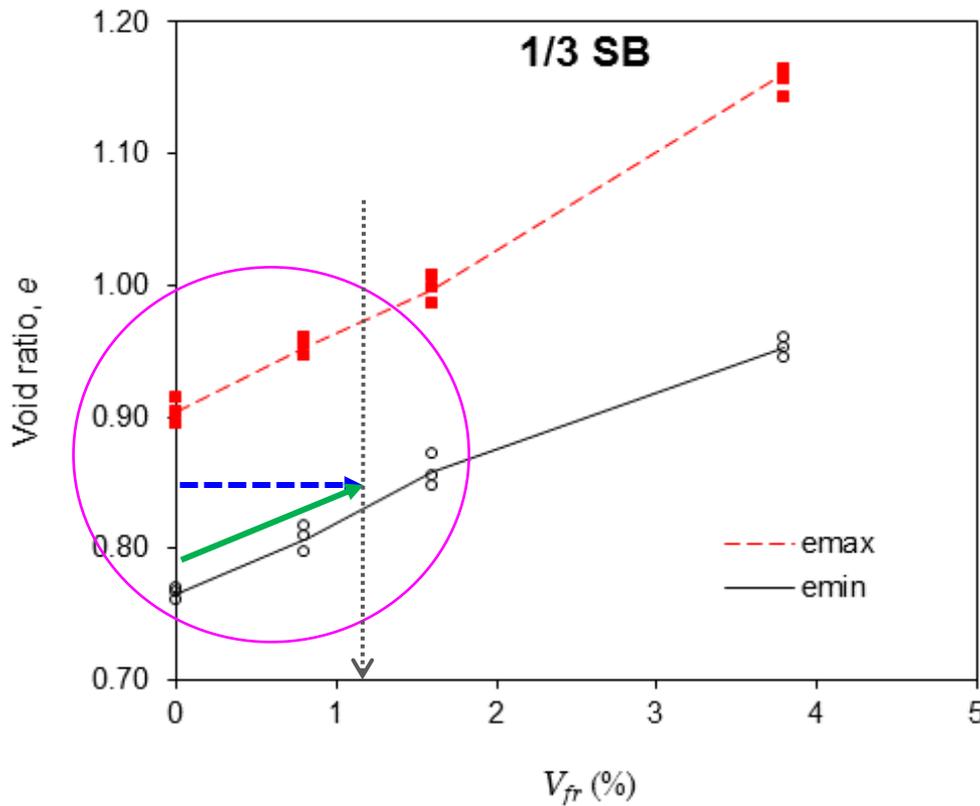
Outline

- Develop a framework for the effects of fibre reinforcements on the **packing** of large aggregates
- Evaluate the **mechanical properties** and **scaling relationships** of fibre reinforced scaled ballast
 - Triaxial tests
 - Image-based deformation measurements
- **Full-scale** laboratory tests using single sleeper tests



Polyethylene fibres

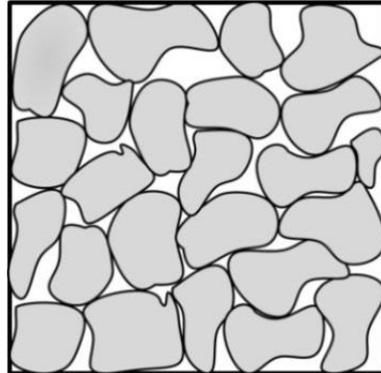
- Scaled ballast use in railway ballast research validated (Le Pen et al. 2013; Sevi 2008)
- Used in railway research (e.g. Sevi et al. 2009; Ishikawa et al. 2011; and Le Pen et al. 2014)



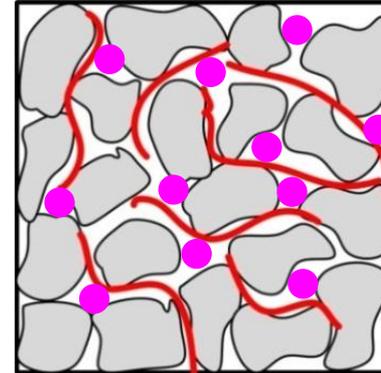
- Volumetric fibre ratio, $V_{fr} = \frac{V_f}{V_s}$
- Void ratio, $e = \frac{V_v}{V_s}$

after Ajayi et al. (2014)

- Varying V_{fr} at constant void ratio will produce different relative densities



Unreinforced ballast
@ constant V_T

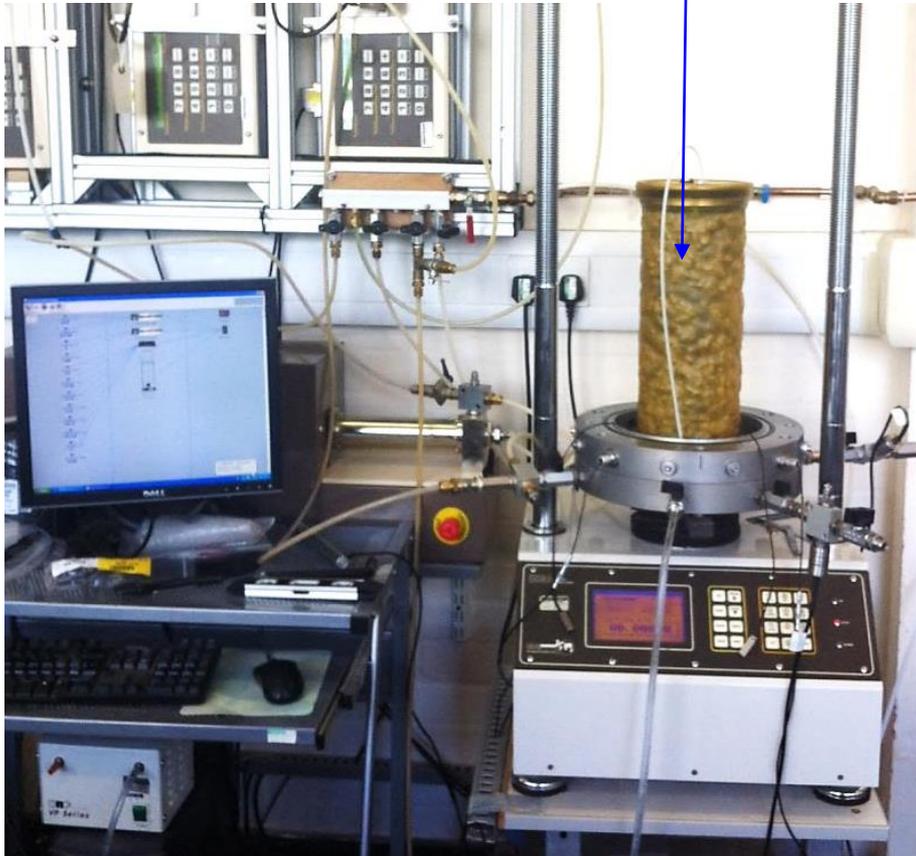


Fibre reinforced ballast
@ constant V_T

- V_s decreases
- V_v increases

- Indication of the behaviour of the constituents of the mixture when fibres are introduced

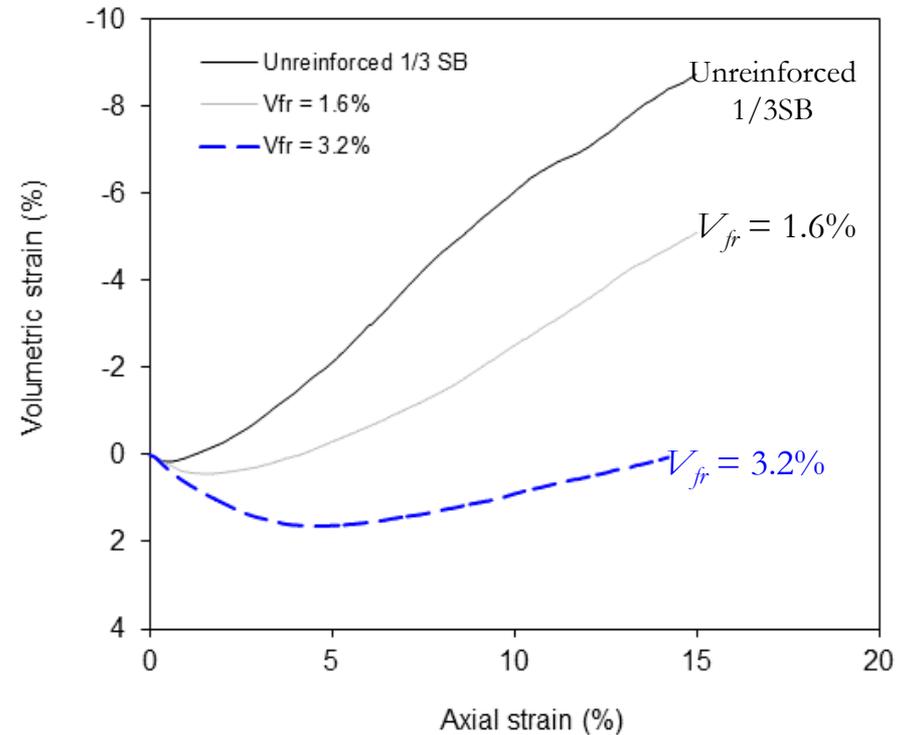
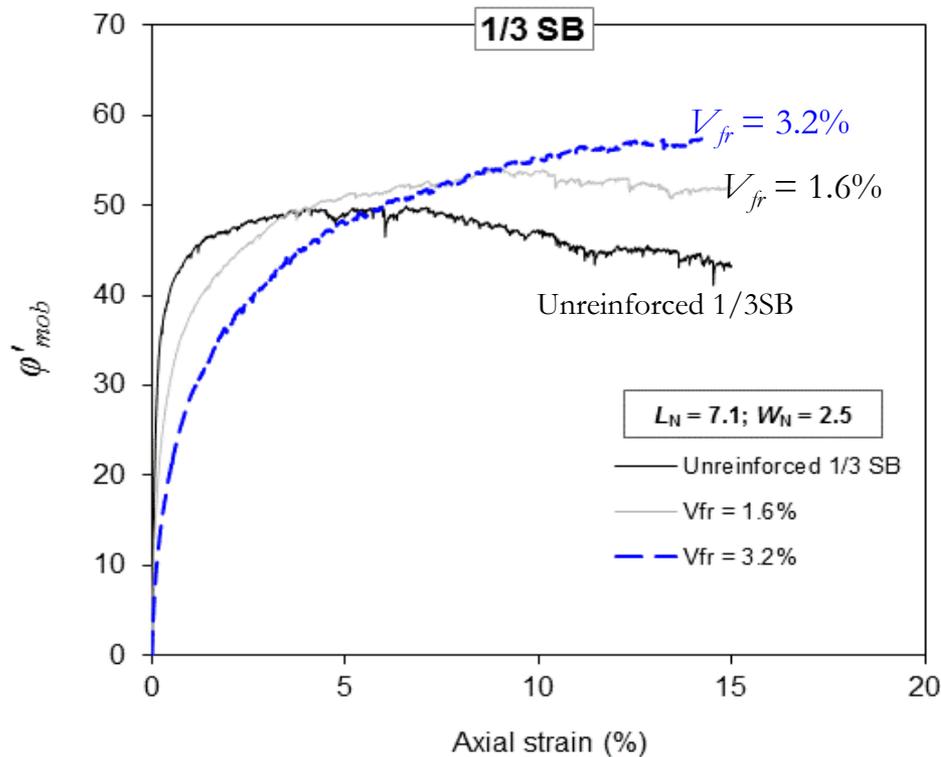
- 150 mm dia.
- 300 mm height



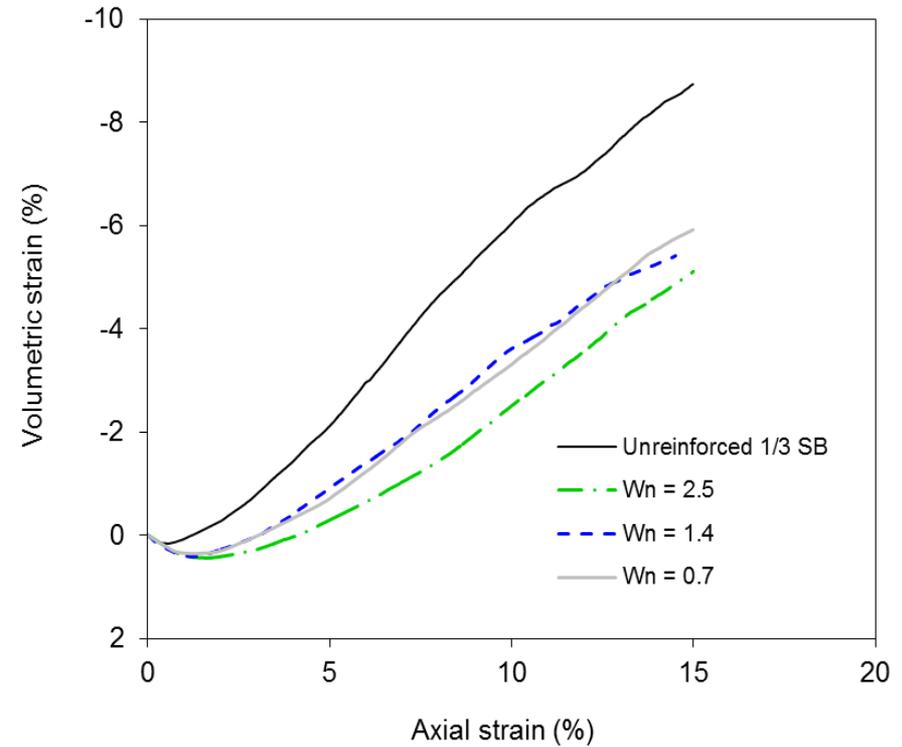
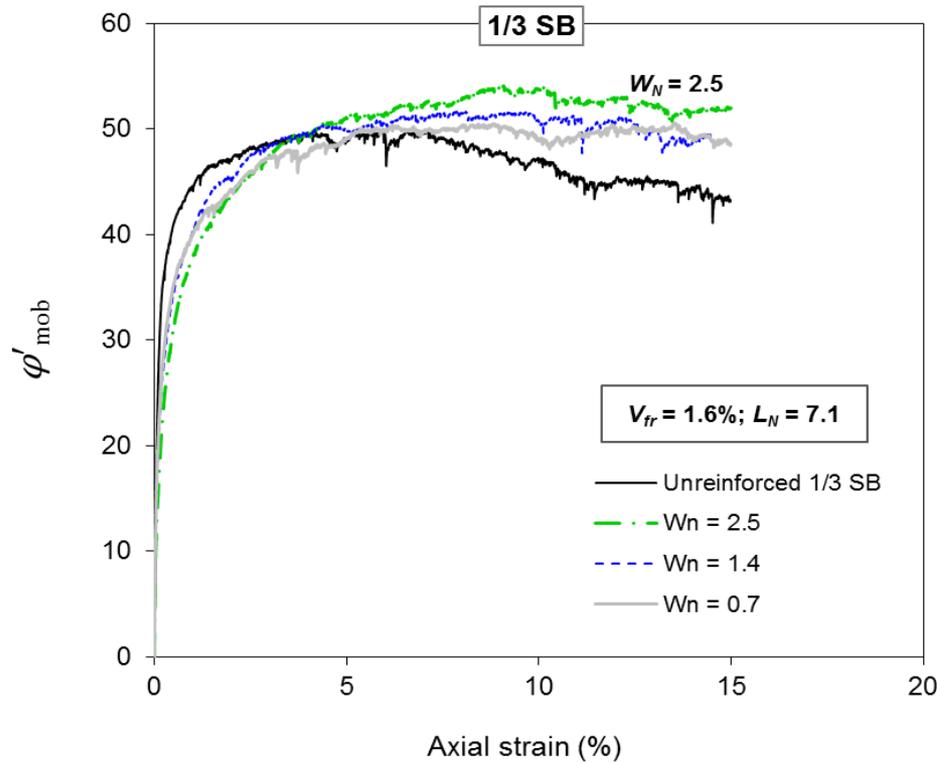
- Confining stress = 30 kPa
- Monotonic loading
- $\sim e_{min}$ (at constant relative density)

Parameters:

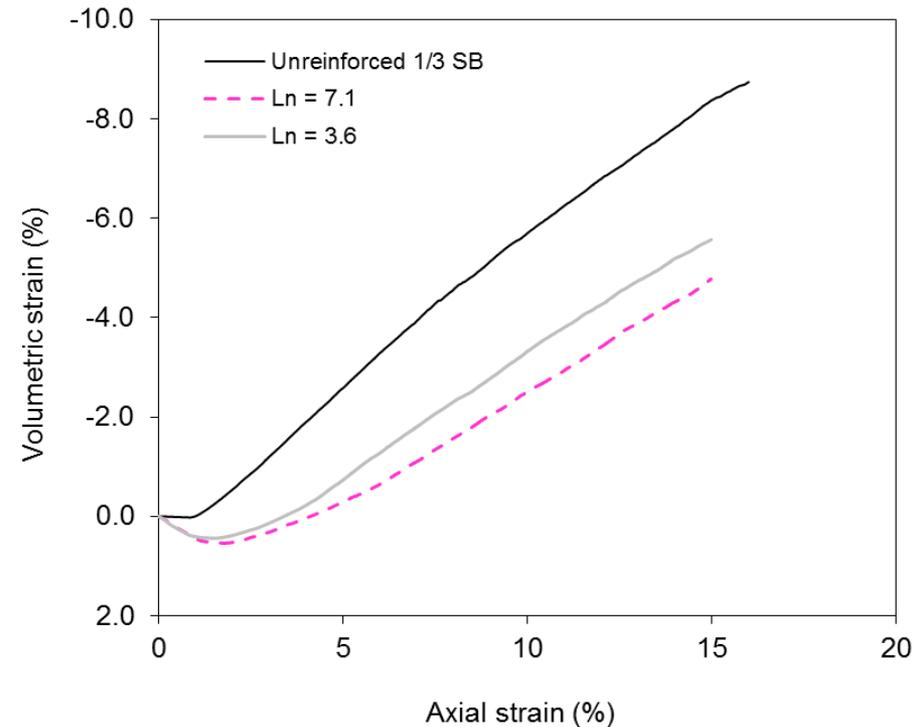
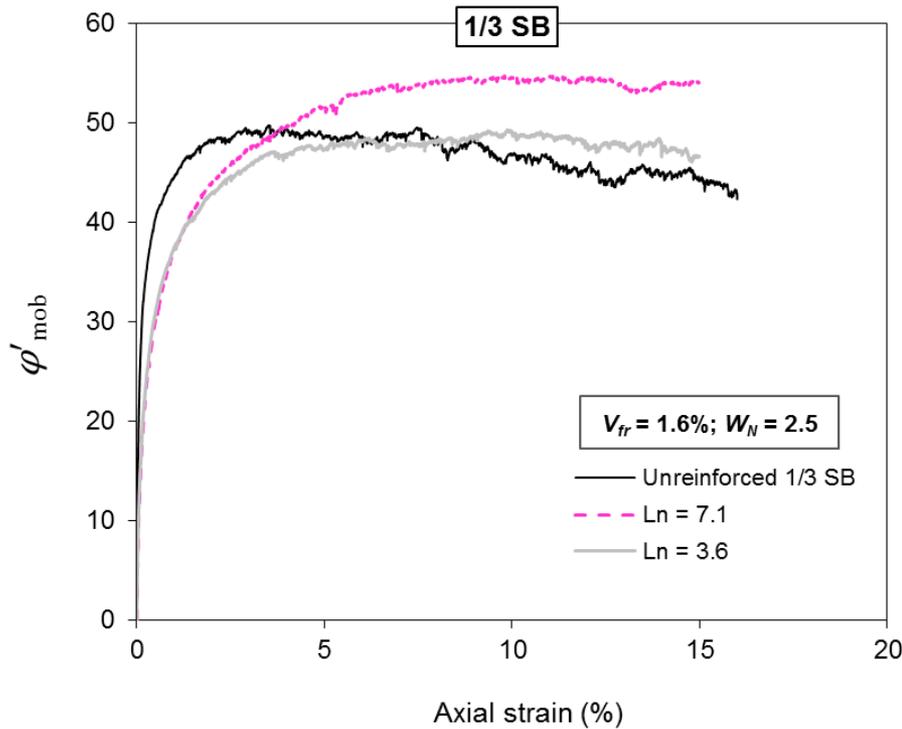
- V_{fr}
- $L_N = \frac{L_f}{D_{50}}$
- $W_N = \frac{W_f}{D_{50}}$

Constant L_N and W_N 

- More ductile
- Reduced initial stiffness
- Suppressed dilation

Constant V_{fr} and L_N 

Constant V_{fr} and W_N



- L_N and W_N both influence mobilised strength.
- At large strains, the influence of L_N on the mobilised strength is more prominent than W_N

Introduction

Density effects

Triaxial tests

Full-scale tests

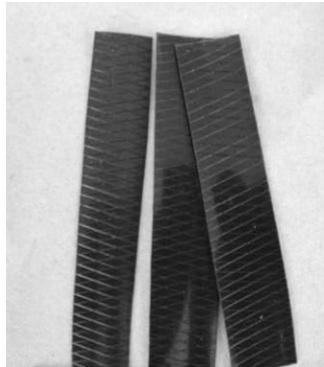
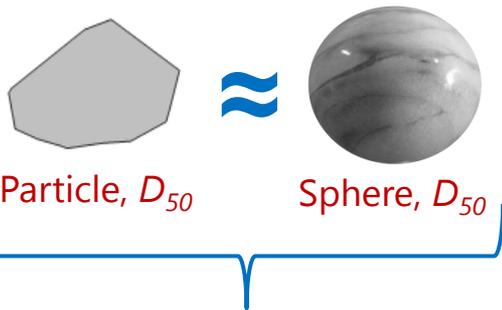
Scaling relationships



$D_{50} \sim 8\text{mm}$

Scaling relationships

$D_{50} \sim 42\text{mm}$



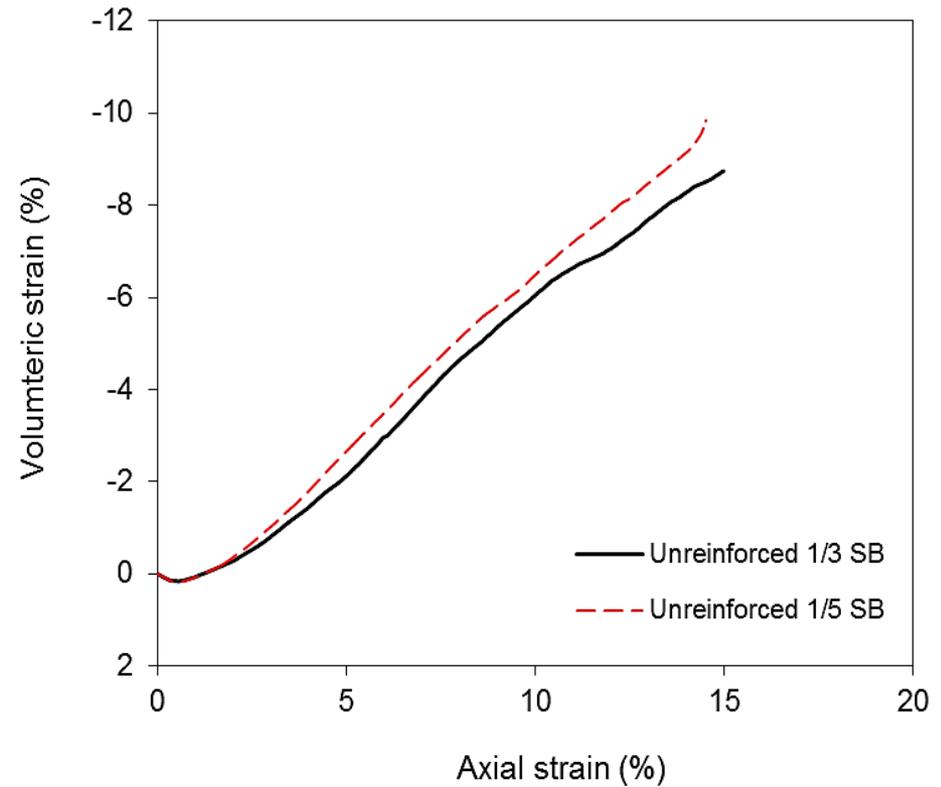
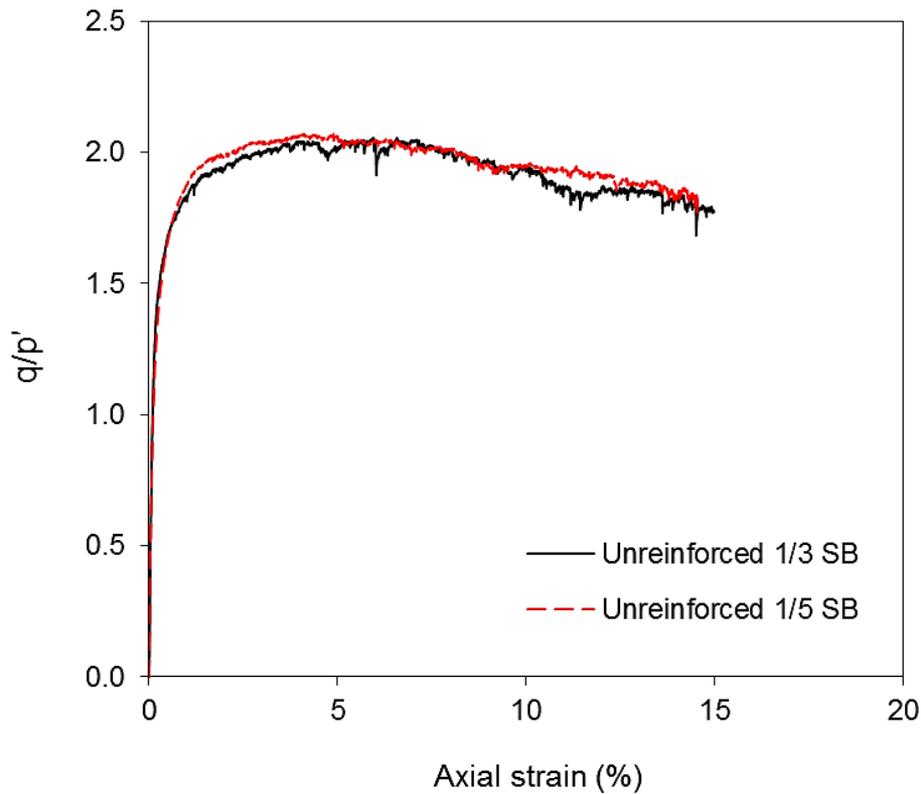
Fibres, N_f

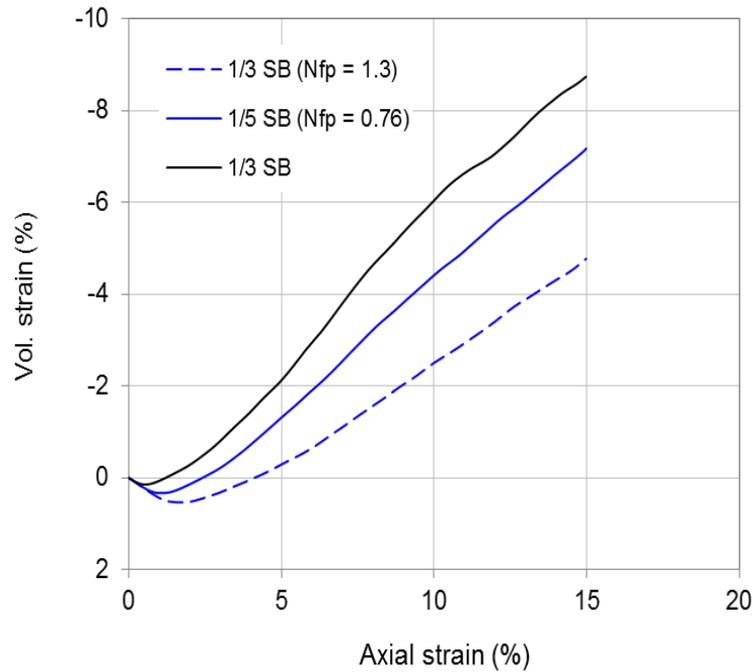
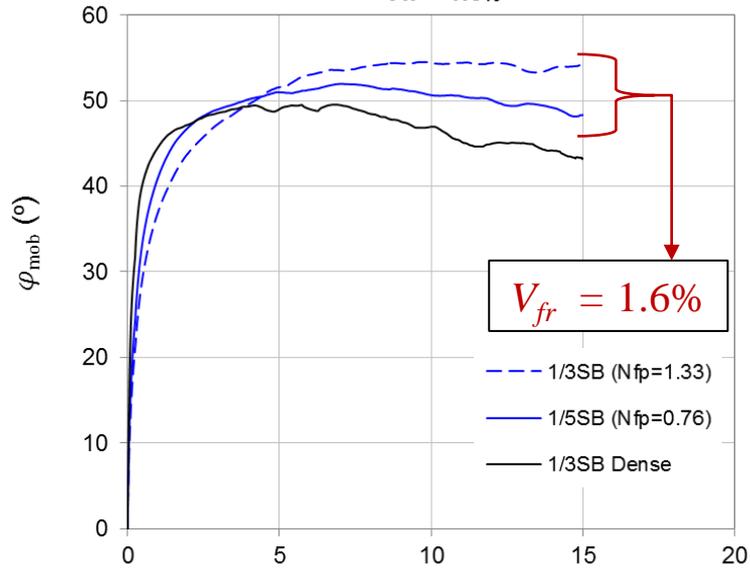
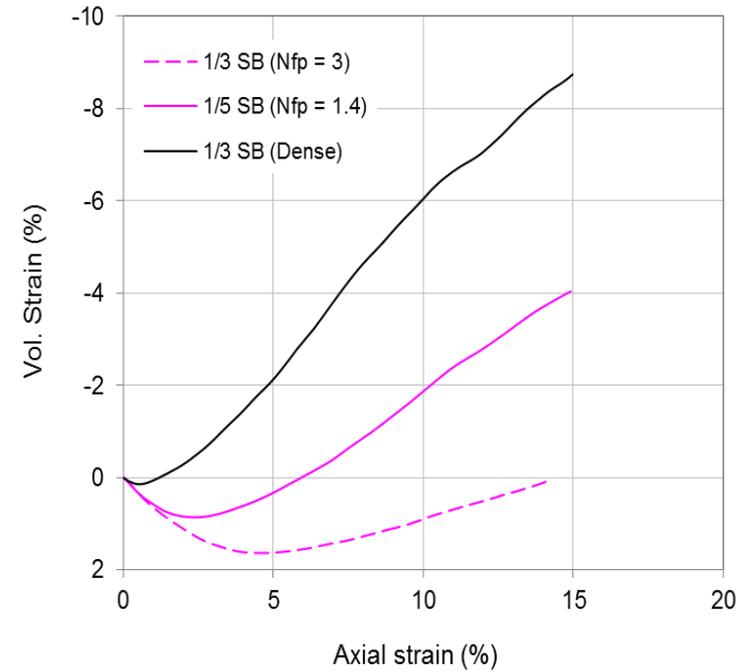
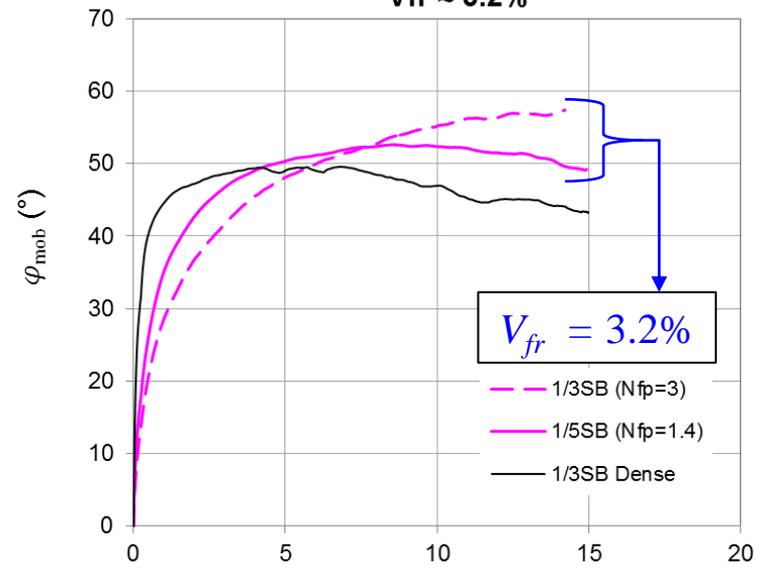
Avg. no. of particles, N_p

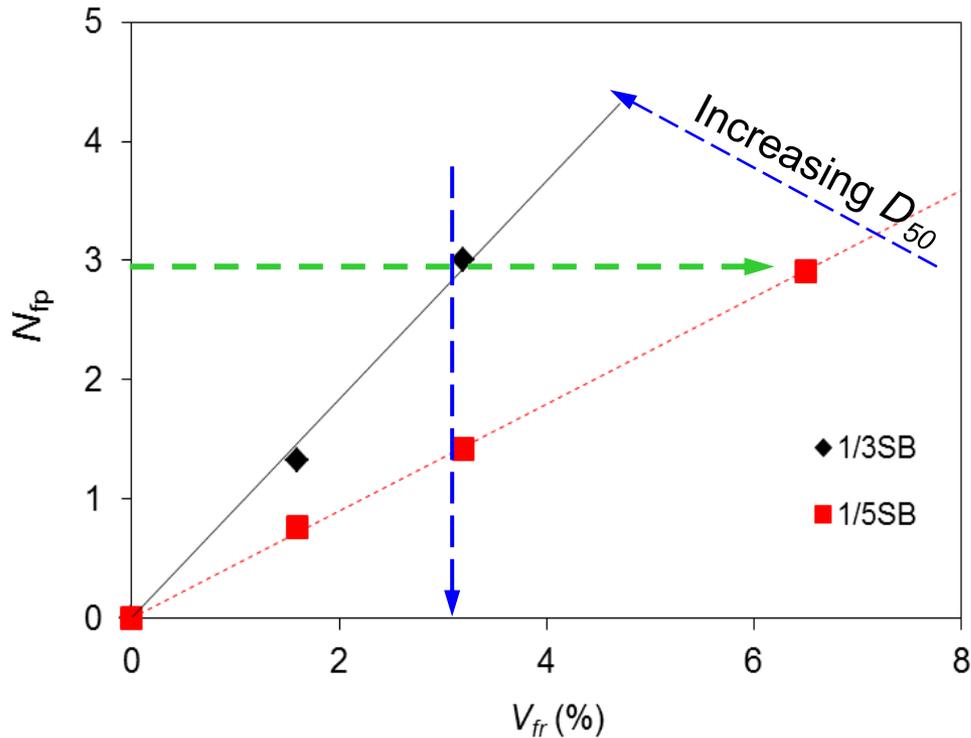
$$N_{fp} = \frac{N_f}{N_p}$$



No bending stiffness:
fibre thickness ignored

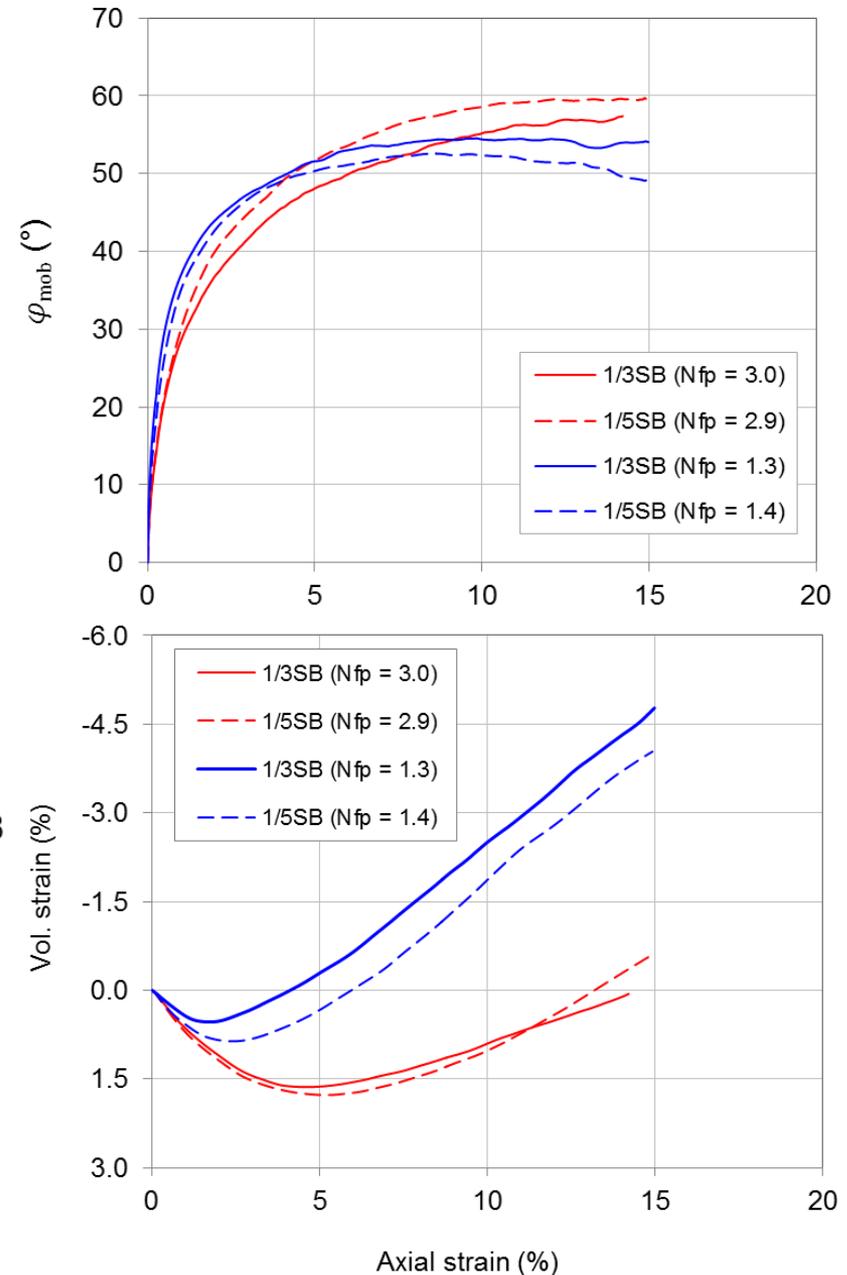
Comparing 1/5th and 1/3rd scaled ballast

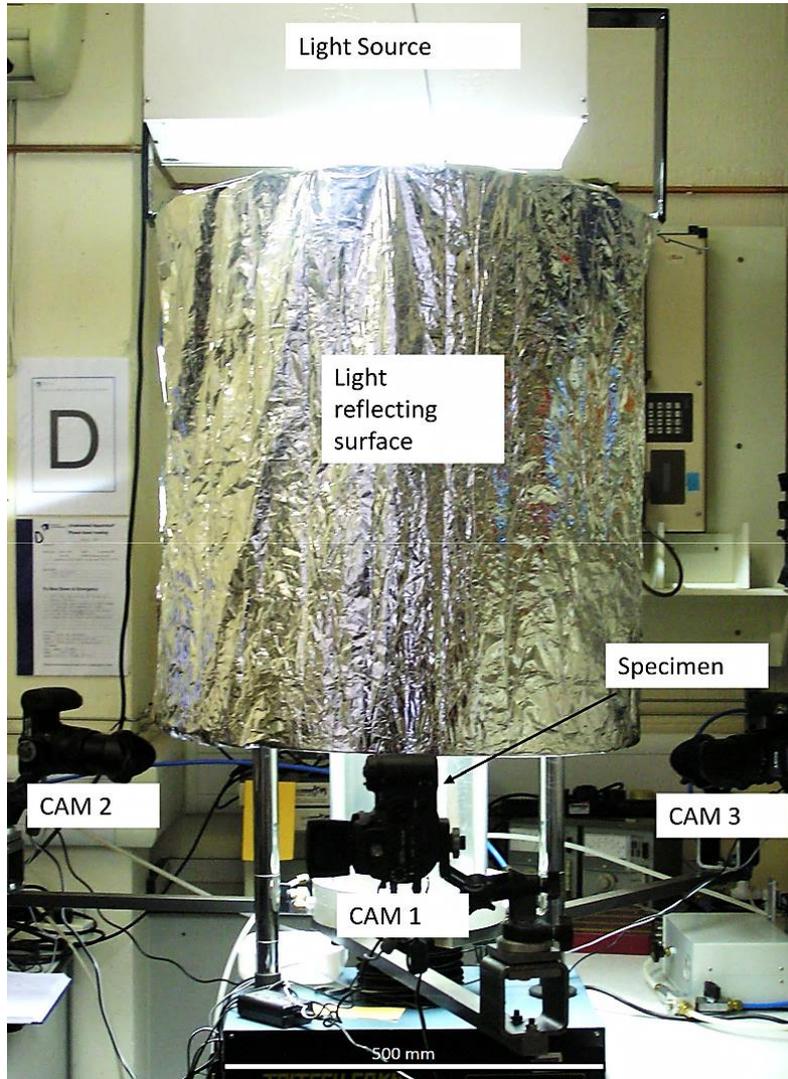
$V_{fr} \approx 1.6\%$  $V_{fr} \approx 3.2\%$ 



- N_{fp} is important when considering fibre/particle interactions over different particle size ranges

* if fibre thickness is not considered





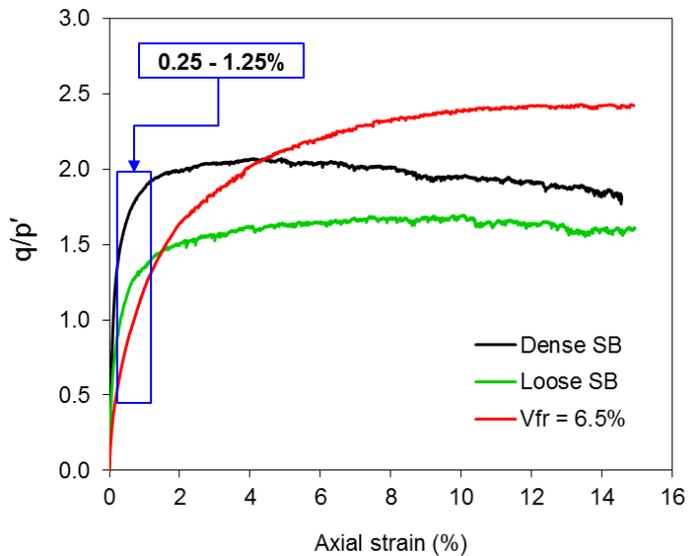
Measurement grids on triaxial specimen

Triax-Digital Image Correlation
(Bhandari et al., 2012)

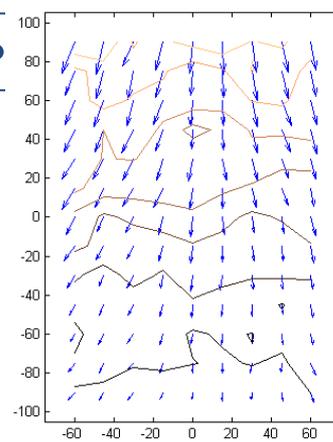
Introduction

Density effects

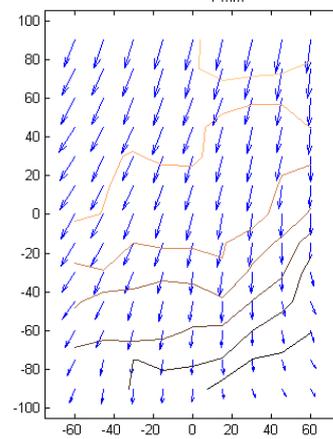
Pre-peak deformation



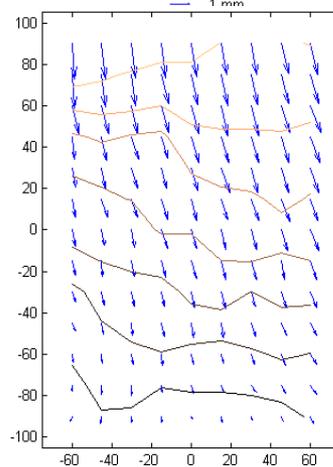
Unreinforced
Dense SB



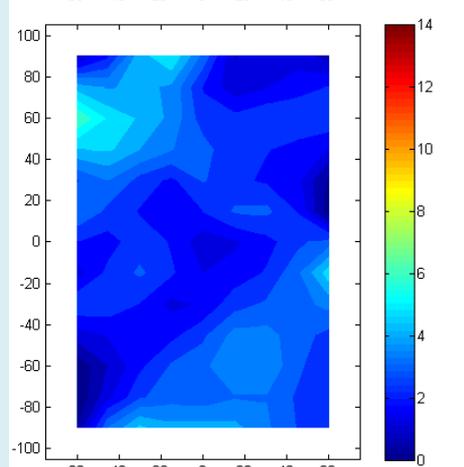
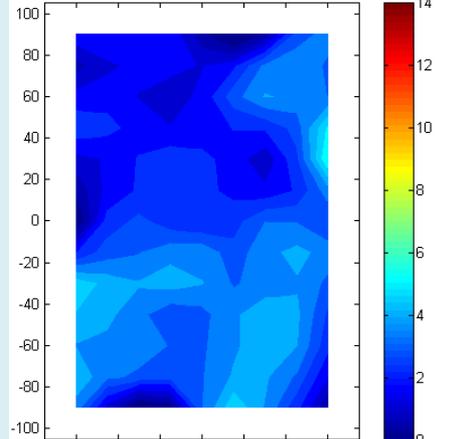
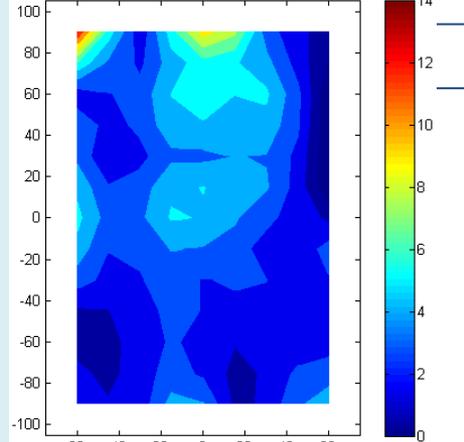
Unreinforced
Loose SB



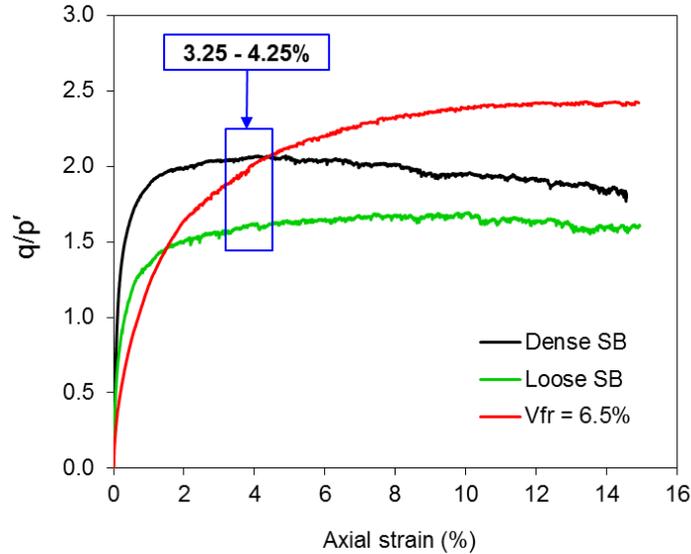
$V_{fr} = 6.5\%$



Maximum shear strain (%)

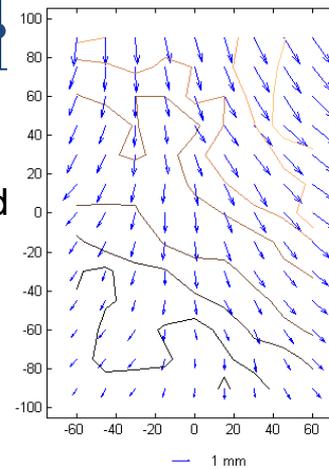
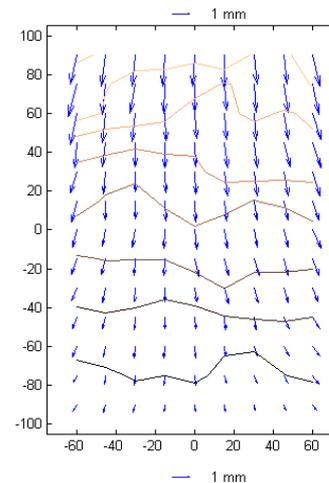
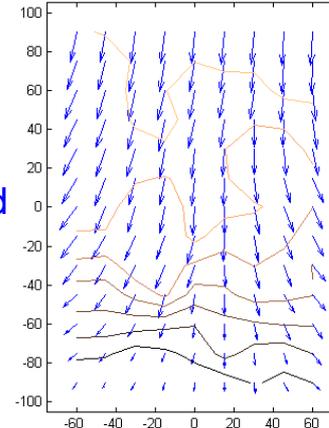


Pre-peak deformation

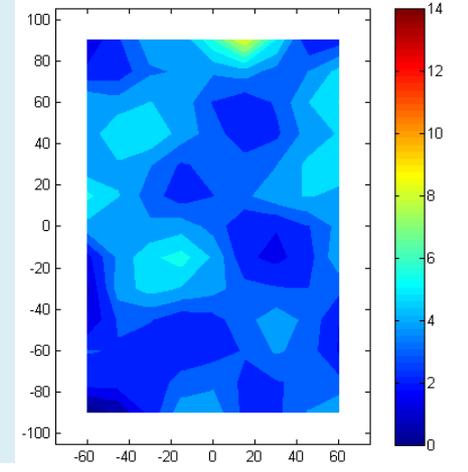
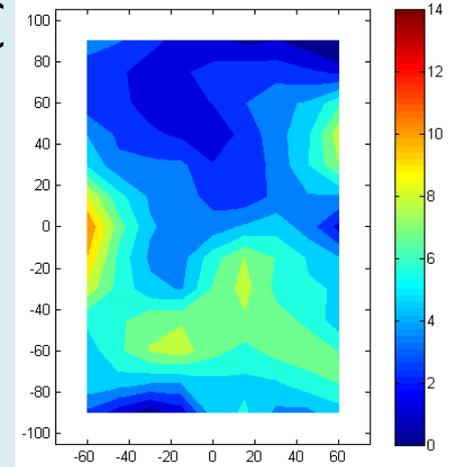
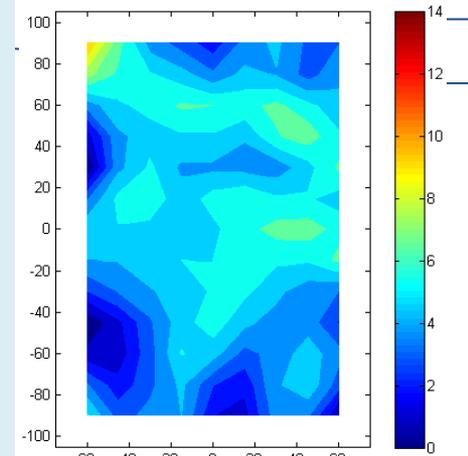


- More uniform vertical deformation in reinforced specimens (i.e. smaller horizontal deformation)
- Smaller and more uniform shear strains in reinforced specimens

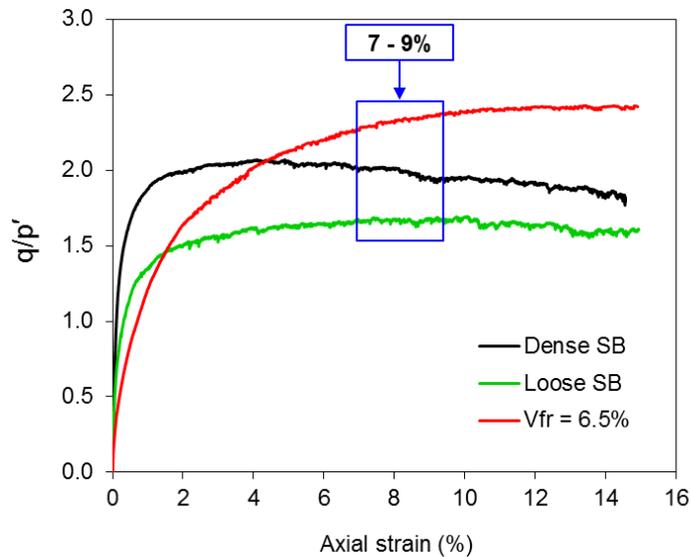
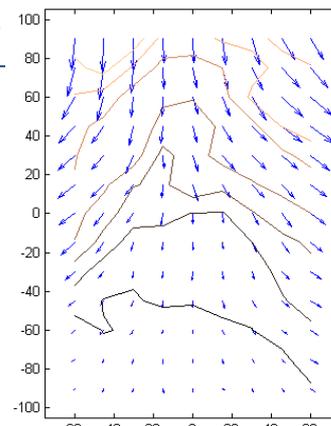
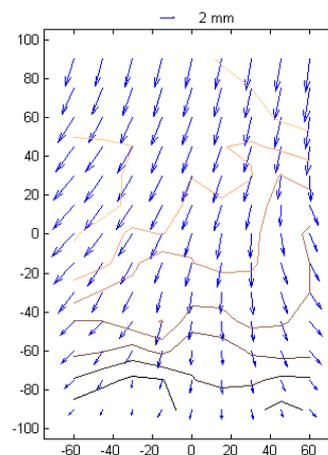
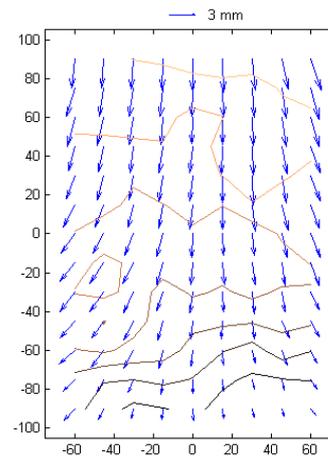
$$V_{fr} = 6.5\%$$

Unreinforced
Dense SBUnreinforced
Loose SB

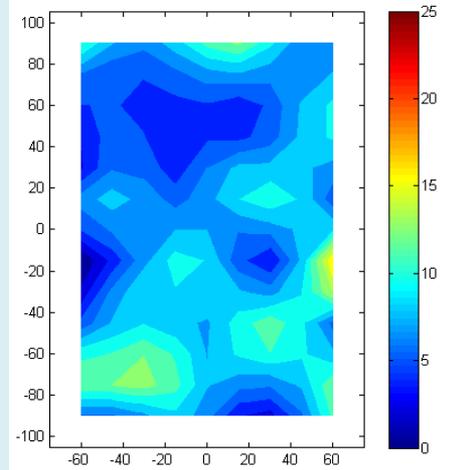
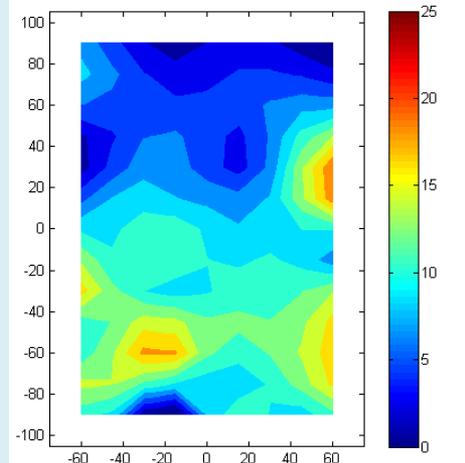
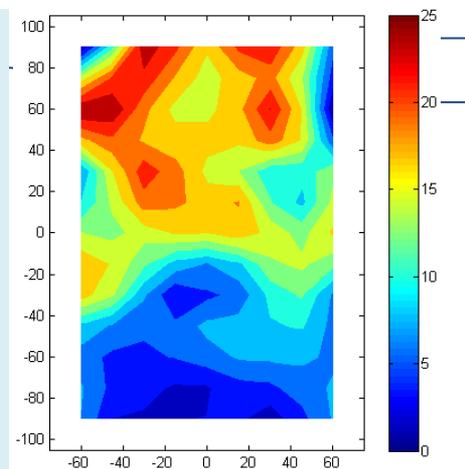
Maximum shear strain (%)



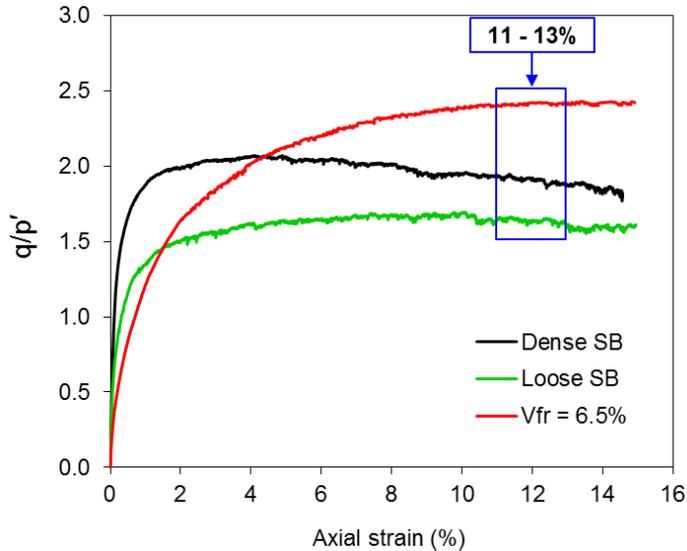
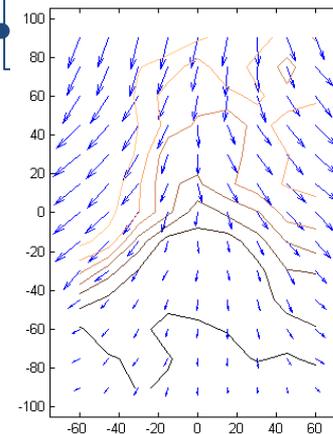
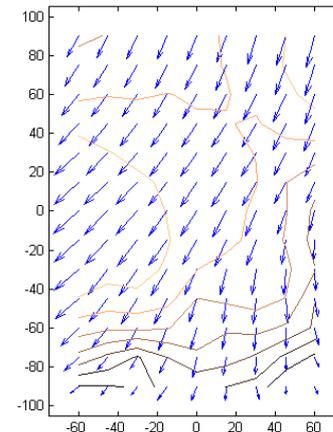
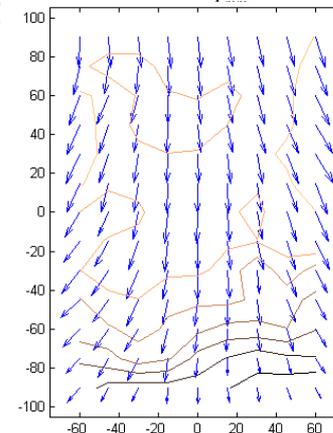
Post-peak deformation

Unreinforced
Dense SBUnreinforced
Loose SB $V_{fr} = 6.5\%$ 

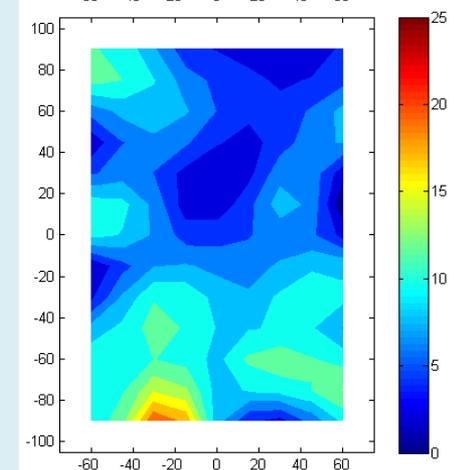
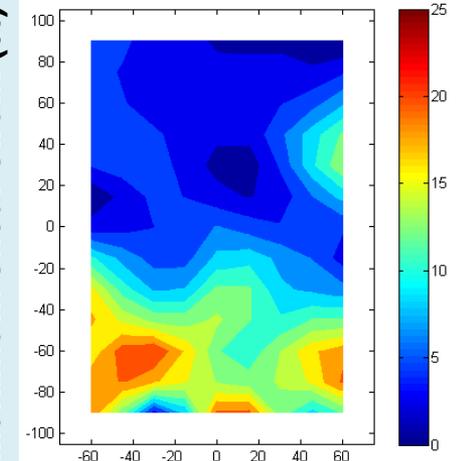
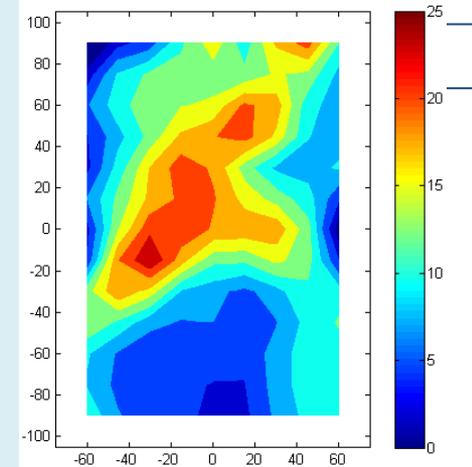
Maximum shear strain (%)



Post-peak deformation

Unreinforced
Dense SBUnreinforced
Loose SB $V_{fr} = 6.5\%$ 

Maximum shear strain (%)



- More homogeneous shear strains in reinforced specimen. Probably due to the reinforcing effect of fibres
- Strain localisation probably evident in bulging deformation and “V” contour shape

Introduction

Density effects

Triaxial tests

Full-scale tests

$$L_N = 7.5$$

$$W_N = 2.5$$

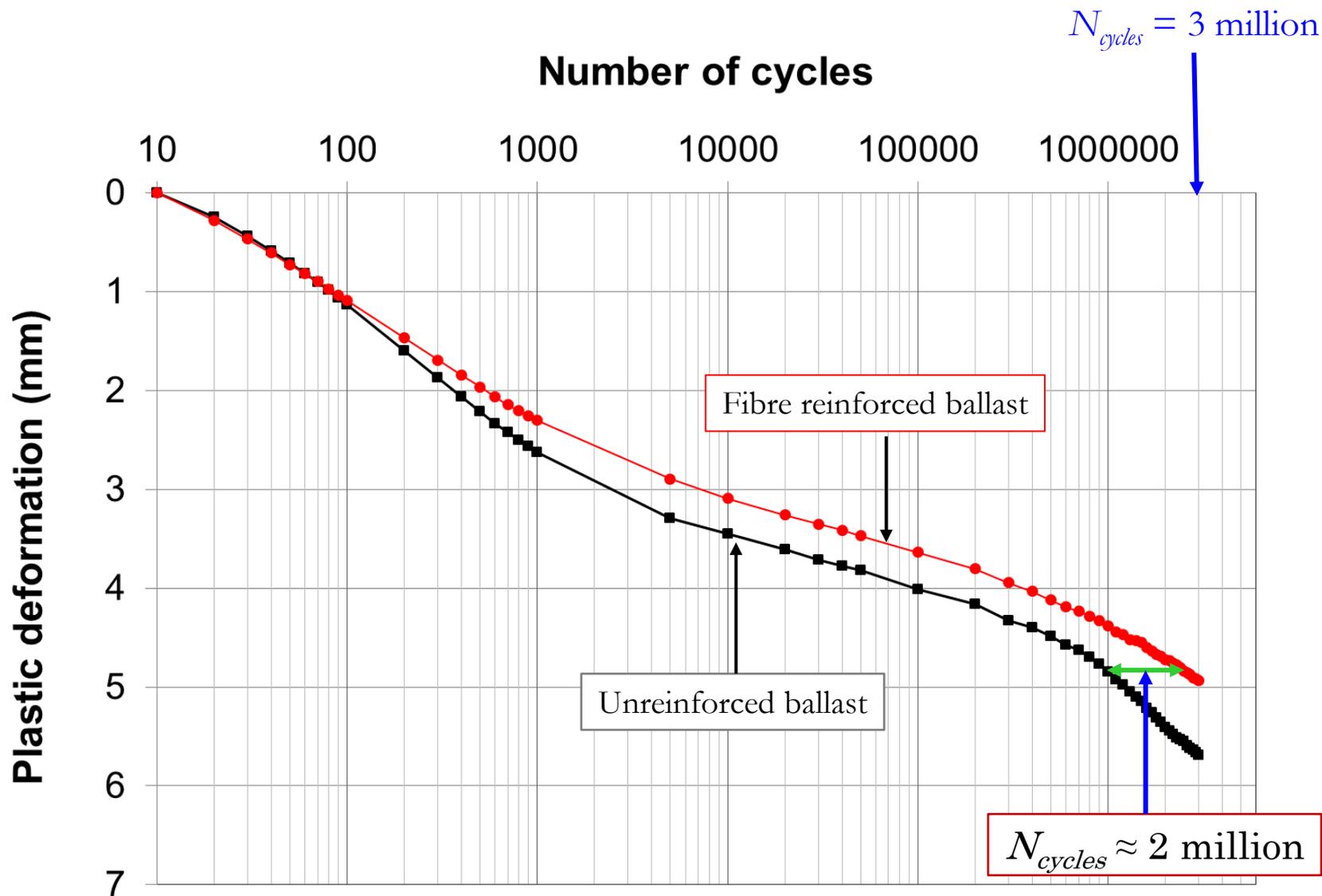
$$N_{fp} = 1.33 \quad (v_{fr} \approx 0.6\%)$$

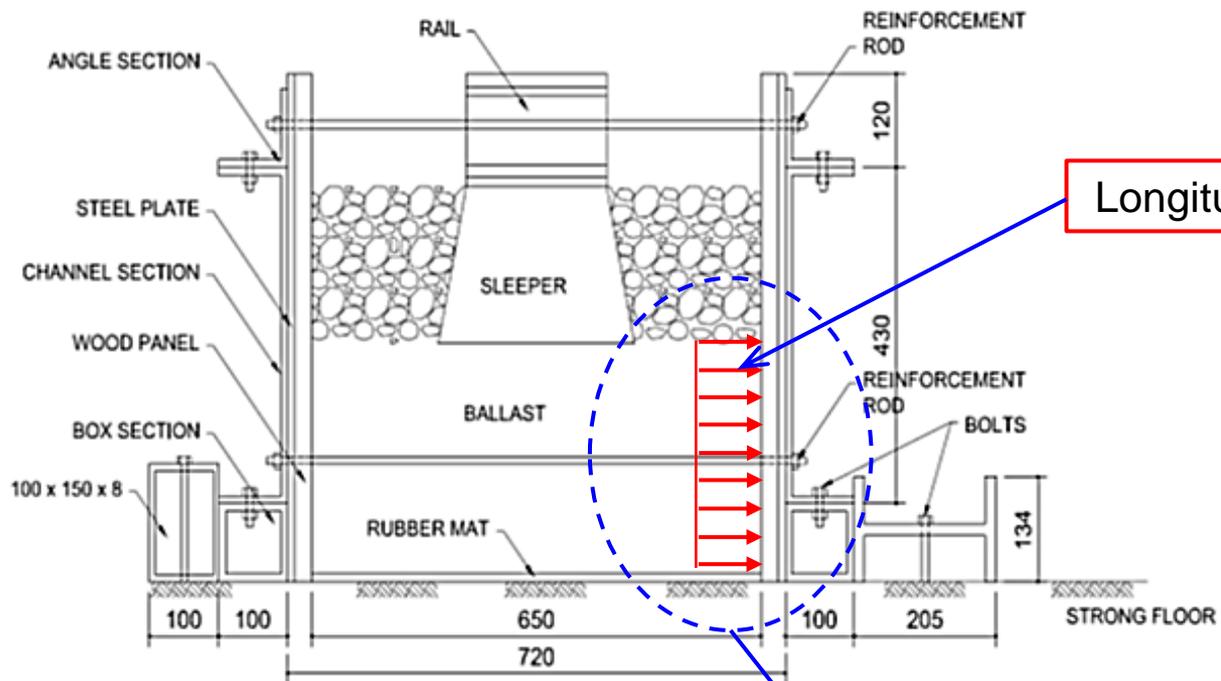


Southampton Railway Test Facility

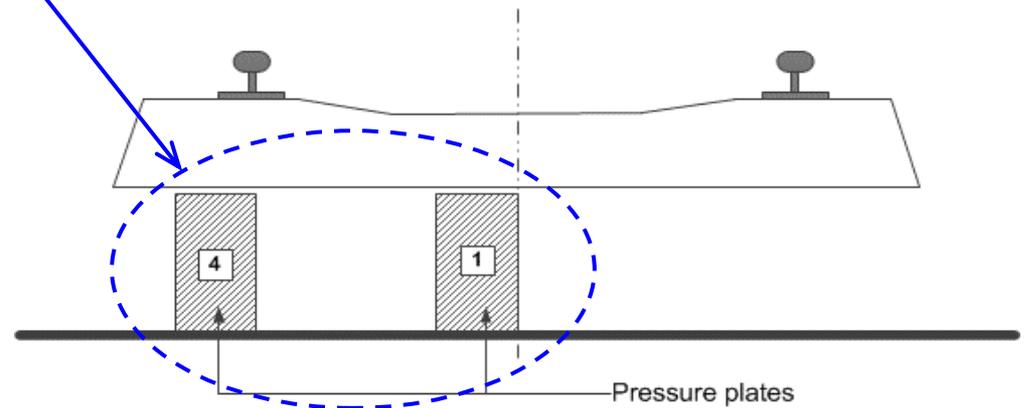


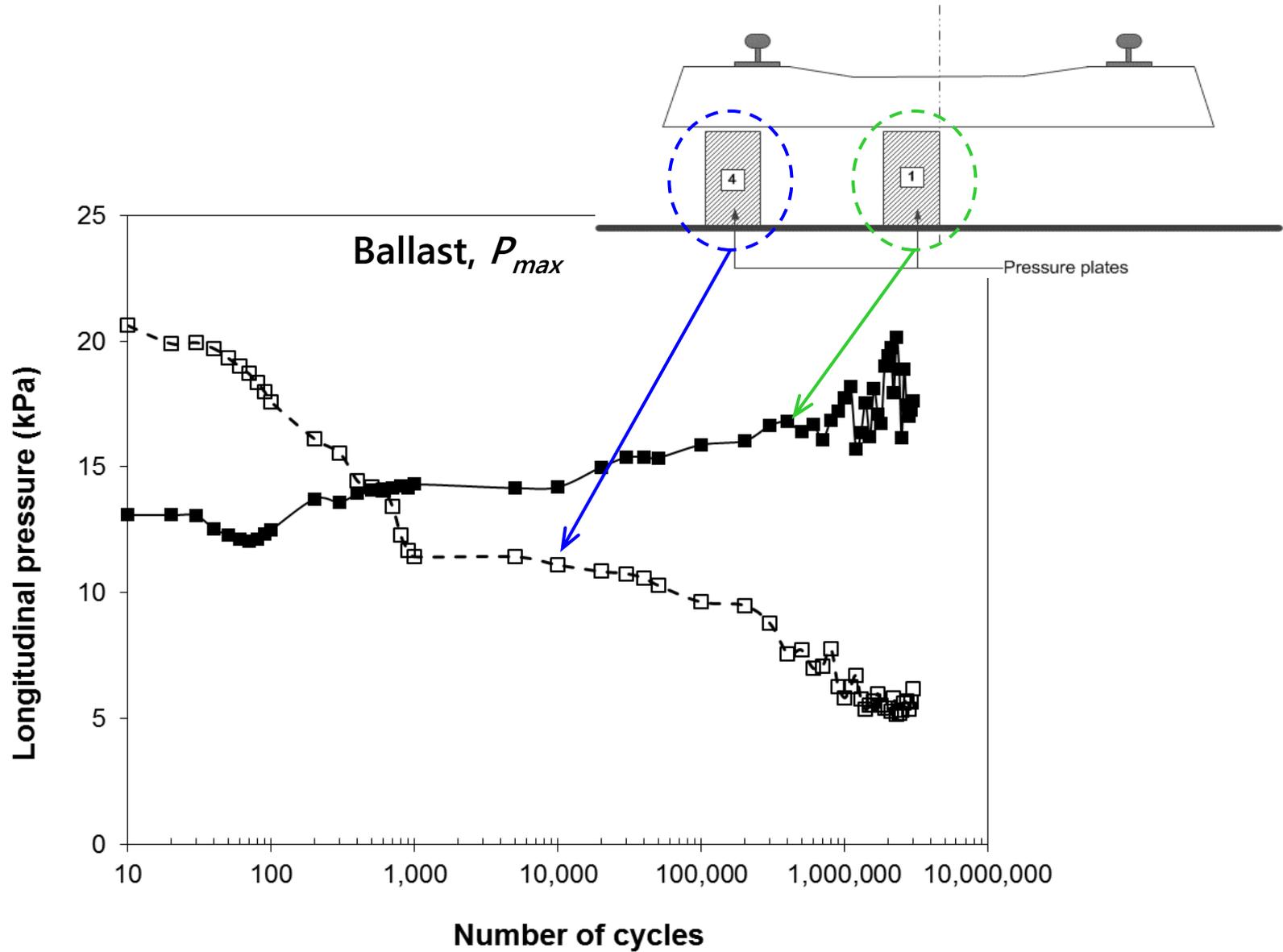
Fibre reinforced ballast

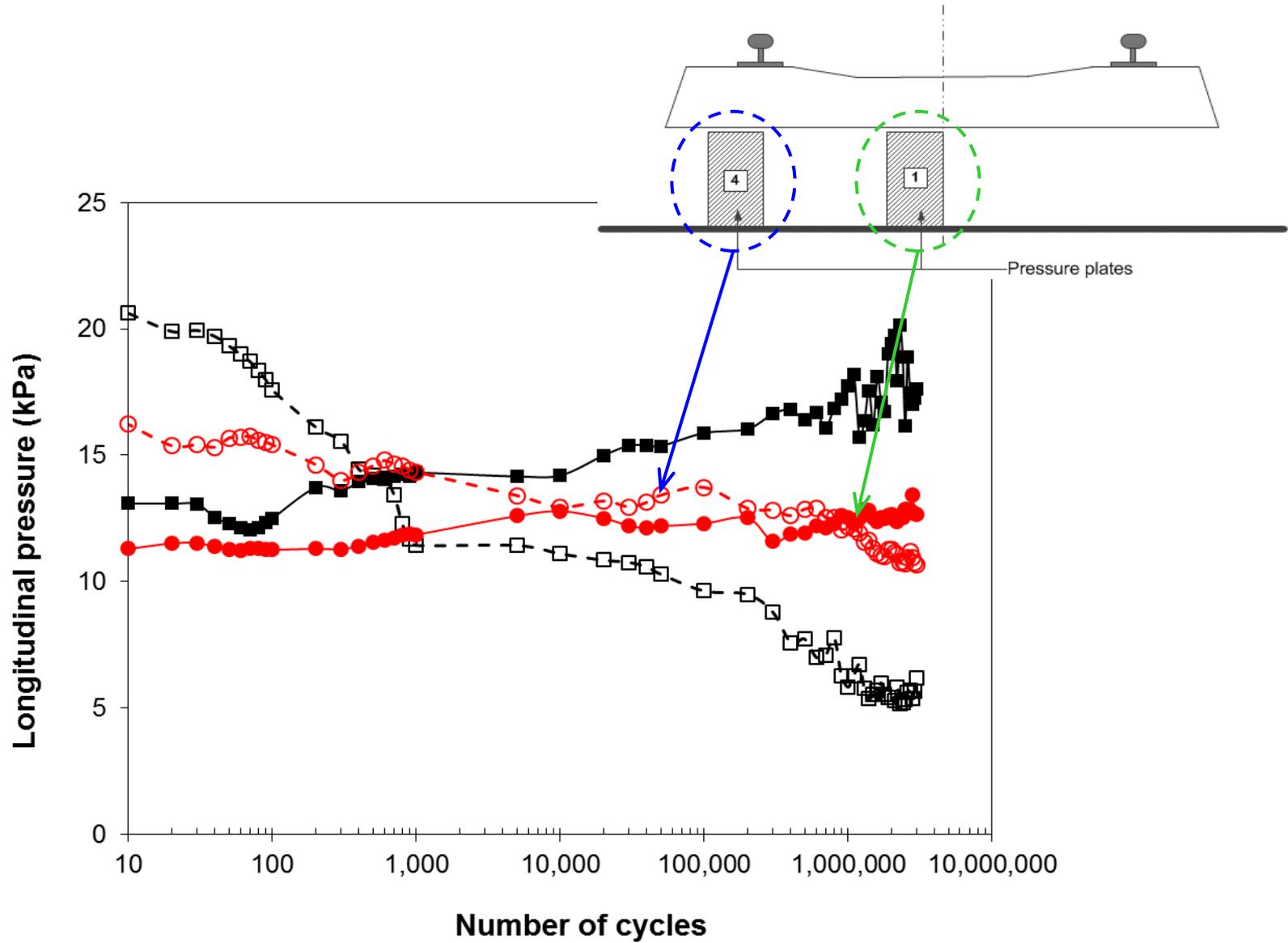




Longitudinal pressure, P







- Addition of fibres to large aggregates increases e_{min} and e_{max} as fibre content (V_{fr}) increases
- Observations from triaxial tests on fibre reinforced scaled ballast
 - Increases ductility
 - Delays dilation
 - Influenced by V_{fr} , L_N , W_N and N_{fp} when considering different particle sizes
 - Produces a more uniformly vertical deformation and homogeneous distribution of shear strain
- Observations from full-scale lab. tests on fibre reinforced railway ballast
 - Reducing plastic settlement
 - More even distribution of longitudinal stresses

Thank you for your attention

Any questions??

References

- Ajayi, O., Le Pen, L.M., Zervos, A. & Powrie, W. (2014). **Effects of Random Fibre Reinforcement on the Density of Granular Materials.** In: SOGA, K., KUMAR, K., BISCONTIN, G. & KUO, M. (eds.) *Geomechanics from Micro to Macro*. University of Cambridge, Cambridge UK: CRC Press/Balkema.
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- Sevi, A.F. (2008). **Physical Modeling of Railroad Ballast Using the Parallel Gradation Scaling Technique within the Cyclic Triaxial Framework.** PhD Thesis, Missouri Univ. of Science and Technology, Rolla, MO.