

# The role of constitutive models in large deformation MPM simulations

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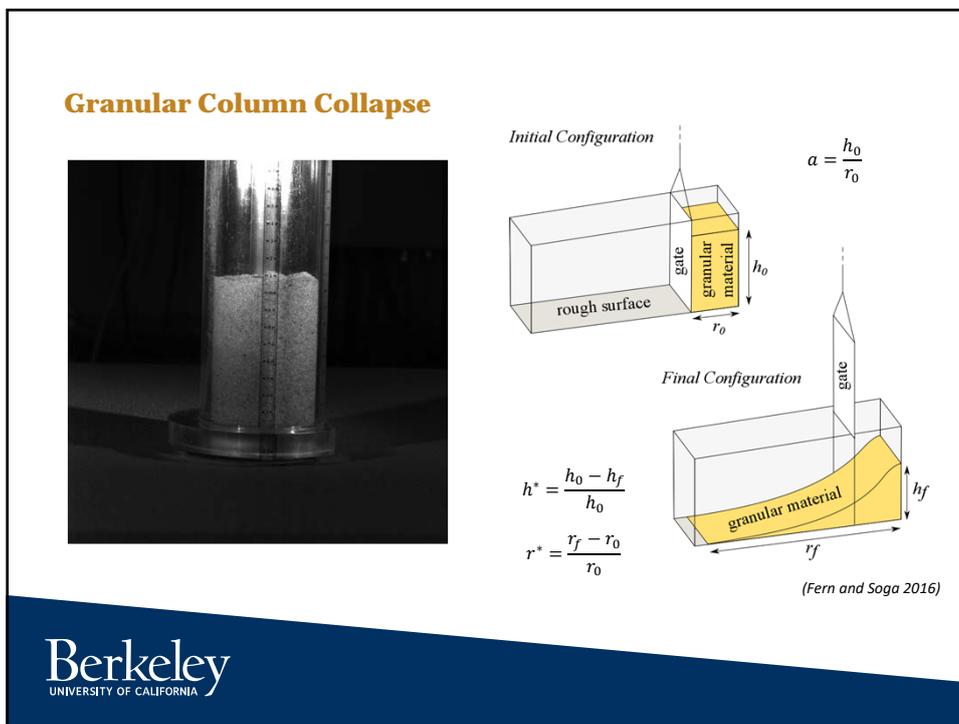
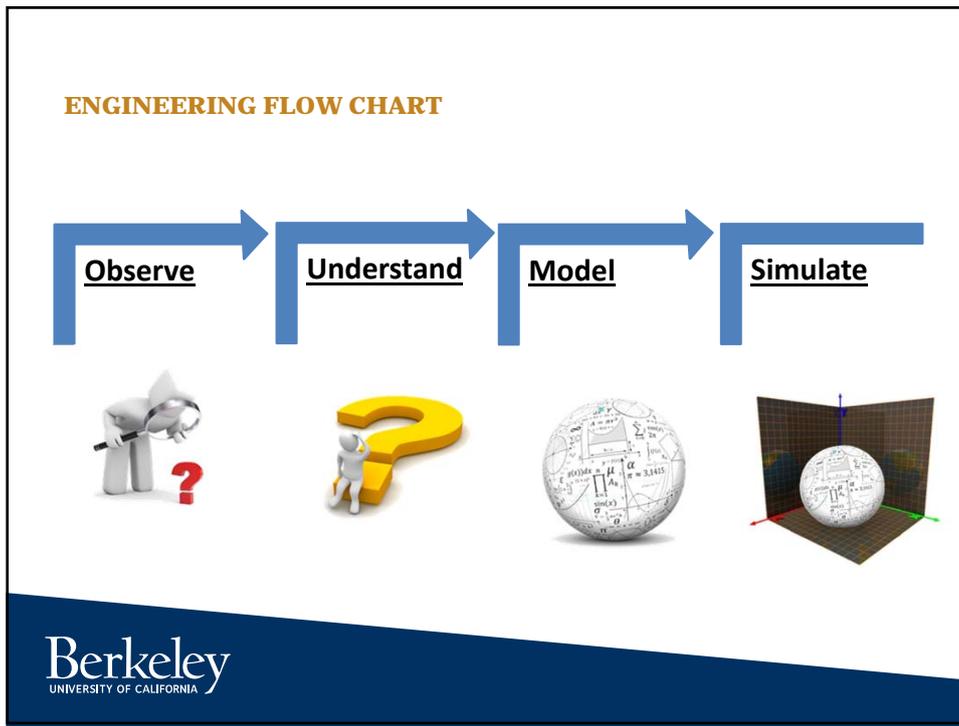
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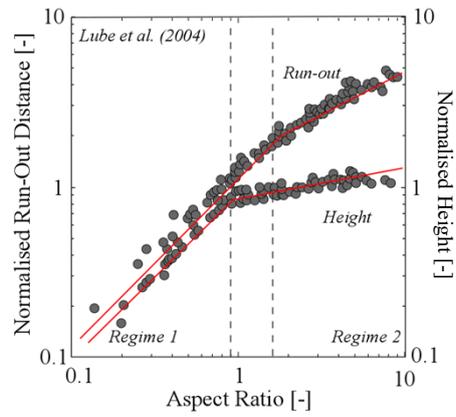
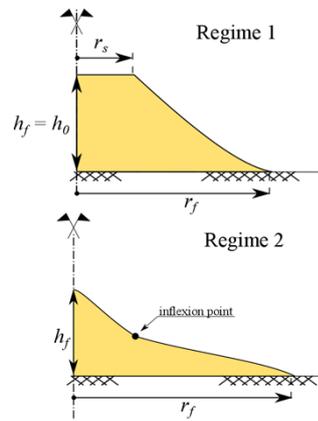
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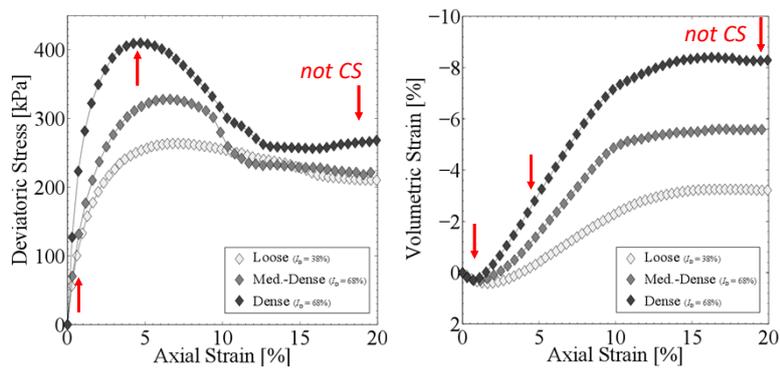
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### Granular Column Collapse



### MECHANICS OF SAND



Dry pluviated Chiba sand (Fern 2016)

**MOHR-COULOMB** (Terzaghi 1943)

$$\tau = c' + \sigma'_N \cdot \tan \phi'$$

Option 1: Small Deformation

$$\phi' = \phi'_{max}$$

$$\psi = \psi_{max}$$

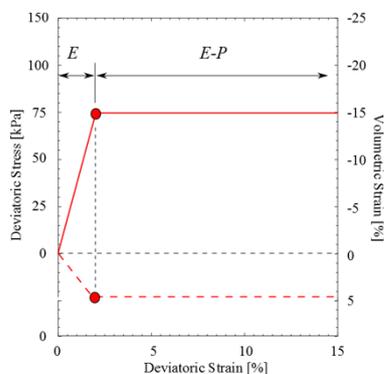
$\psi \neq 0$   
causes unrealistic results

Option 2: Large Deformation

$$\phi' = \phi'_{cs}$$

$$\psi = 0$$

$$(e \neq e_{cs})$$



Does the small strain behaviour play a role in the large deformation simulations?



**MOHR-COULOMB STRAIN SOFTENING** (Terzaghi 1943, Abbo and Sloan 1995)

$$\tau = c' + \sigma'_N \cdot \tan \phi'$$

$$\phi'_{max} \xrightarrow{E_d^p \rightarrow \infty} \phi'_{res}$$

$$\psi_{max} \xrightarrow{E_d^p \rightarrow \infty} \psi_{res}$$

$$c'_{max} \xrightarrow{E_d^p \rightarrow \infty} c'_{res}$$

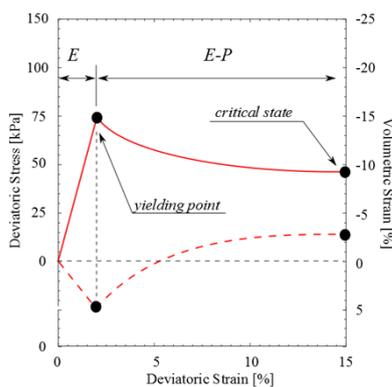
Small Deformation

$$\psi = \psi_{max} \rightarrow \phi' = \phi'_{max}$$

Large Deformation

$$\psi = 0 \rightarrow \phi' = \phi'_{cs}$$

$$e \neq e_{cs}$$



Does the absence of energy dissipation in the 'elastic hardening' phase play a role?



**Nor-Sand** (Jefferies 1993)

**Stress-Dilatancy rule**

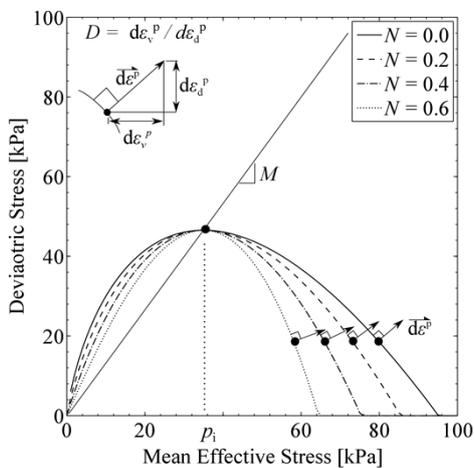
$$\eta' = M + (N - 1)D$$

**Yield Surface**

$$F = \eta' - \frac{M_\theta}{N} \left[ 1 + (N - 1) \left( \frac{p_i}{p'} \right)^{\frac{N}{1-N}} \right]$$

**Hardening rule**

$$\frac{\partial p_i}{\partial \varepsilon_d} = H \cdot \dots \cdot (p_{i,max} - p_i)$$



**Nor-Sand** (Jefferies 1993)

**Maximum Yield Surface**

$$p_{i,max} = p' \left( 1 + D_{min} \frac{N}{M} \right)^{\frac{N-1}{N}}$$

$\uparrow$   
 $D_{min} = \chi \cdot \Psi$   
 $\uparrow$   
 $\Psi = e - e_c$

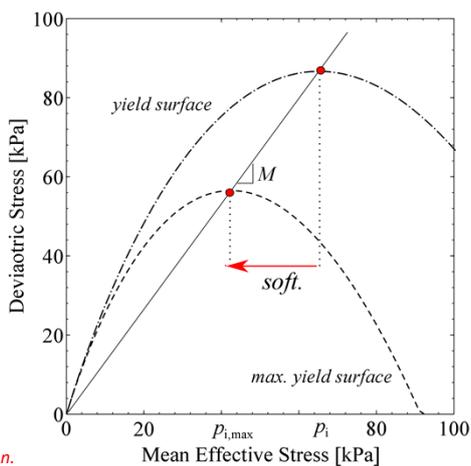
hardening  $\rightarrow p_i < p_{i,max}$

softening  $\rightarrow p_{i,max} < p_i$

critical state  $\rightarrow p' = p_i = p_{i,max}$

$\Psi = 0 \rightarrow e = e_{cs}$

Causes instabilities at large deformation.



### Material Point Method

1. Map MP info to nodes
2. Solve balance equations
3. Map velocity field to MPs
4. Update position of MPs

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MPM Research Community  
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void → solid → [Material Point]

● Material point    □ Node  
◇ Continuum body    □ Grid / Cell

*(Fern and Soga, 2016a)*

### Granular Column Collapse

**Column 1**  
● Monitored MP

**Column 2**  
● Monitored MP

$p'_0 = 20 \text{ kPa}$

Legend:  
○ MC  
□ MCSS loose  
■ MCSS dense  
◇ NS loose  
◆ NS dense

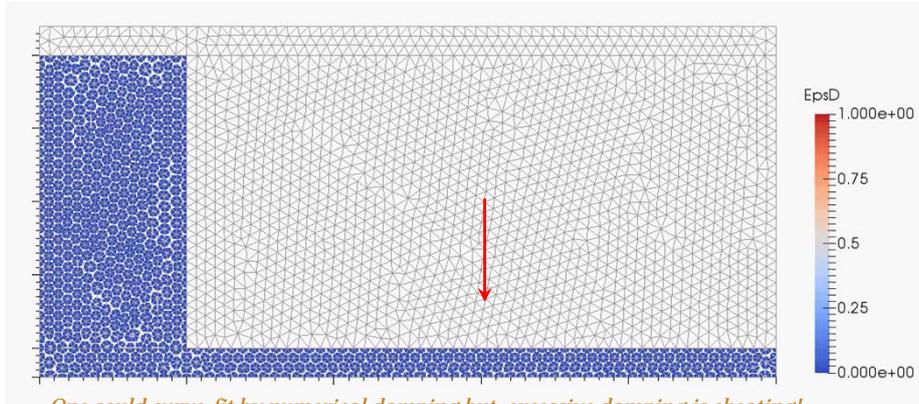
Y-axis: Deviatoric Stress [kPa]  
X-axis: Axial Strain [%]

Y-axis: Volumetric Strain [%]  
X-axis: Axial Strain [%]

*(Fern and Soga, 2016a)*

### Granular Column Collapse

*Mohr-Coulomb (critical state parameters)*

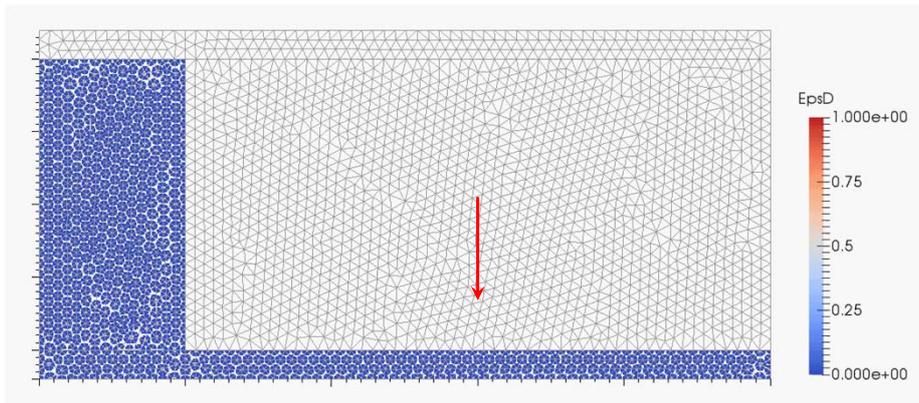


*One could curve-fit by numerical damping but, excessive damping is cheating!*



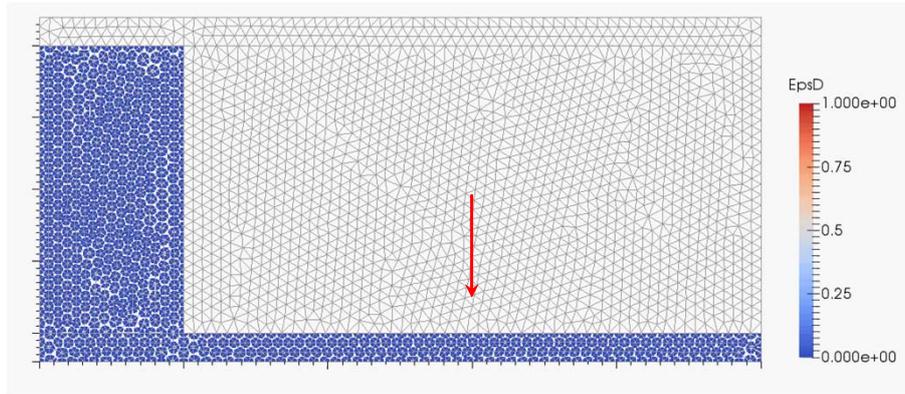
### Granular Column Collapse

*Mohr-Coulomb Strain Softening – loose sand*



### Granular Column Collapse

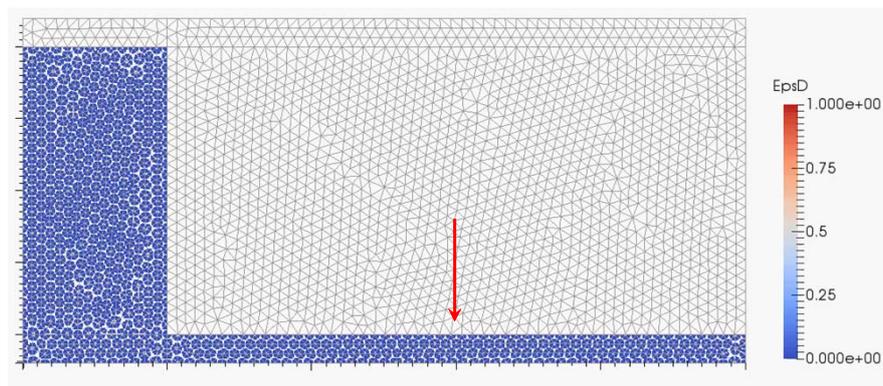
*Mohr-Coulomb Strain Softening – dense sand*



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### Granular Column Collapse

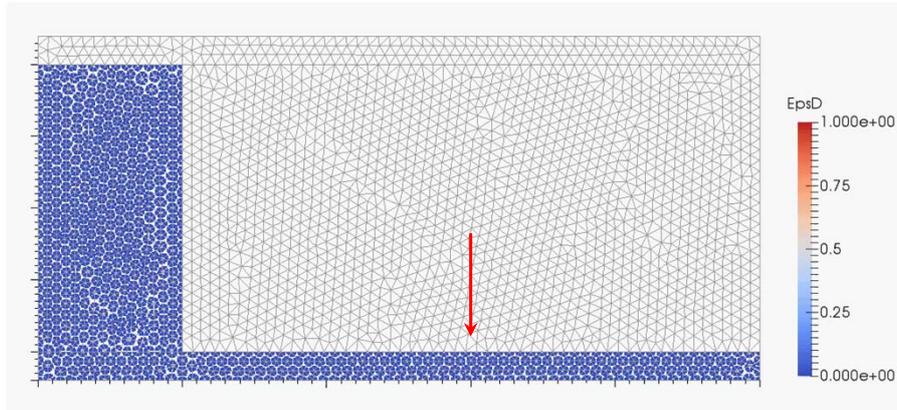
*Nor-Sand – loose sand*



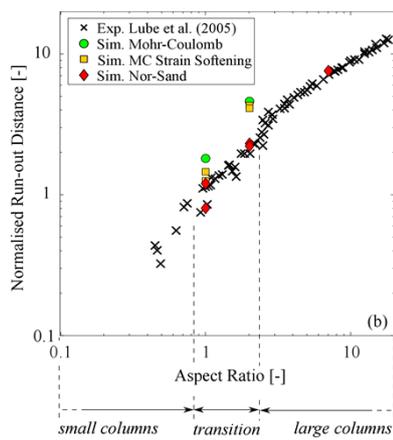
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### Granular Column Collapse

Nor-Sand – dense sand



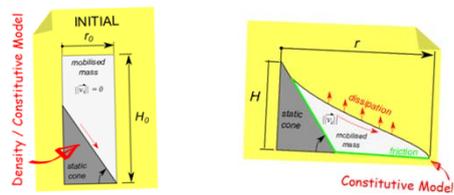
### Granular Column Collapse



$$E_{pot}^{mob} = \dot{E}_{kin}^{mob} + E_{pot}^{mob} + \text{dissipation}$$

$E_{pot}^{mob}$  (initial)      Kinematic energy of mobilised mass → velocity  
 Potential energy of mobilised mass  
 - Initial aspect ratio  
 - peak state (constitutive model)

Dissipation of energy  
 - basal and shear band friction  
 - flow rule (constitutive model)



### Energy Considerations

#### Small columns

$$E_{pot}^{mob} = m_{mob} \cdot g \cdot h_{mob}^{CG}$$

$$= \frac{1}{3} h_0^3 \cot \varphi' (1 - n) \rho_s g$$

#### Large columns

$$E_{pot}^{mob} = m_{tot} \cdot g \cdot h_{tot}^{CG} - m_{stat} \cdot g \cdot h_{stat}^{CG}$$

$$= \left( \frac{1}{2} h_0^2 r_0 - \frac{1}{6} r_0^3 \tan^2 \varphi' \right) (1 - n) \rho_s g$$

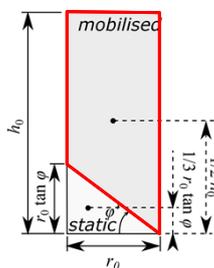
#### Failure angle (Bolton, 1986)

$$\varphi' = \varphi'_{cs} + 0.8 \psi$$

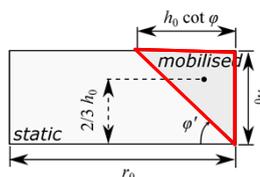
$$\psi = \alpha \cdot I_R$$

$$I_R = \frac{e_{max} - e}{e_{max} - e_{min}} \ln \left( \frac{Q}{p'} \right) - R$$

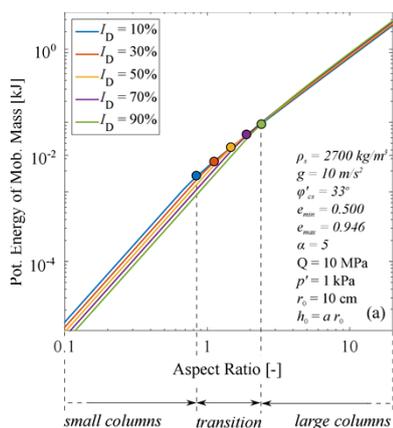
#### Large Aspect Ratios



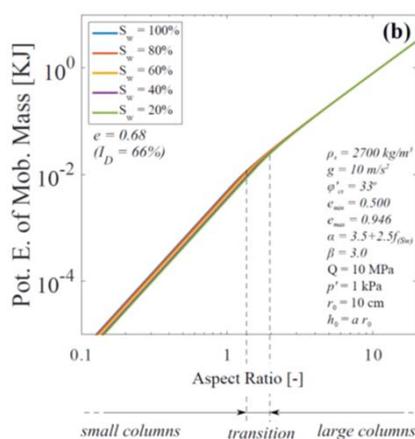
#### Small Aspect Ratios



### Granular Column Collapse



(Fern and Soga, 2016a)



(Fern and Soga, 2017)

## Conclusion

- **The constitutive model has a two-fold influence.**
  - **It defines the failure surface** (hence the mobilised energy)
    - Therefore, the small strain behaviour cannot be neglected from 'large deformation simulations'
  - **and it controls the energy dissipation** (hence the run-out distance)
    - Which becomes increasingly important with large deformation
- **The initial density influences the constitutive model**
  - indirectly through an enhancement of the model parameters (i.e. Mohr–Coulomb)
  - or directly through its inclusion as model variable (i.e. Nor-Sand.and)
- **The requirements and implementations of constitutive models for large deformation MPM simulations can differ from small deformation FE simulations.**
- **The choice of the constitutive model plays a major role in the predictions of the failure** (i.e. type of failure, run-out distance) and its choice is not trivial.

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