



UNIVERSITÁ DI SALERNO (ITALY)

Department of Civil Engineering

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Mechanisms and consequence of bed entrainment for landslides of the flow type (and countemeasures)

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## ACKNOWLEDGMENTS

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LARAM SCHOOL

LANDSLIDE RISK ASSESSMENT AND MITIGATION - UNIVERSITY OF SALERNO

[www.laram.unisa.it](http://www.laram.unisa.it)

(hosting in September 2016 the 8th Olok Zienkiewicz Course “Geomechanics of landslides”)

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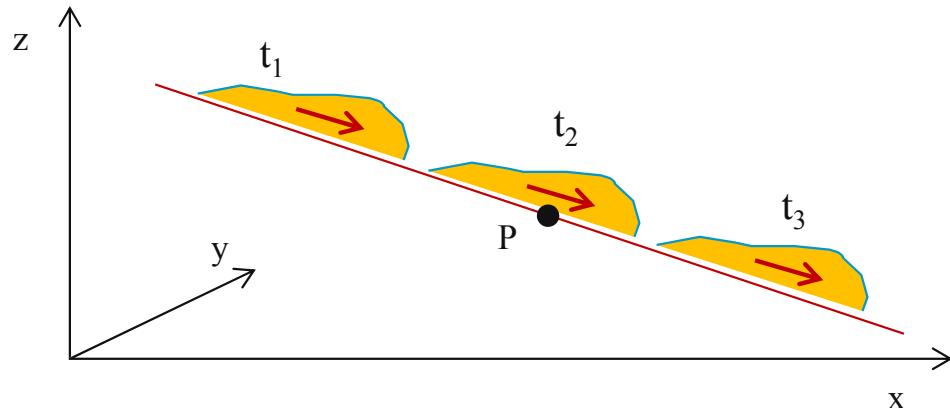
Giuseppe Castorino,

Mario Coppola

## MECHANISMS

# BED ENTRAINMENT

## SCHEME



$$e_r = - \partial z / \partial t \quad e_r: \text{entrainment (or erosion) rate @ P}$$

$$h_{er} = \int_0^{t_{er}} e_r dt \quad h_{er}: \text{erosion height @ P (during } t_{er})$$

$$V_{er} = \int_{A_{er}} h_{er} dx dy \quad V_{er}: \text{volume eroded (in } A_{er})$$

$$V_f = V_i + V_{er} \quad V_f, V_i: \text{final (and initial) volume}$$

$$A_f = V_f / V_i \quad A_f: \text{amplification factor}$$

## EXAMPLE

Tsing Chan event (Hong Kong, 2001)



} small triggering volume  
( $V_i = 345 \text{ m}^3$ )

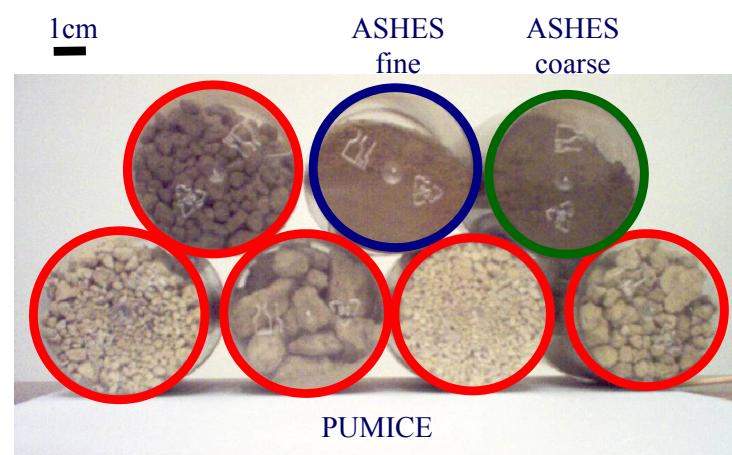
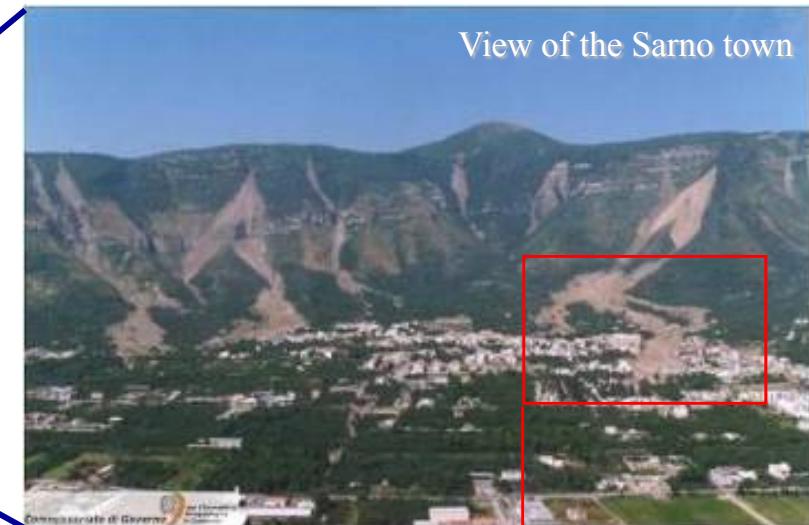
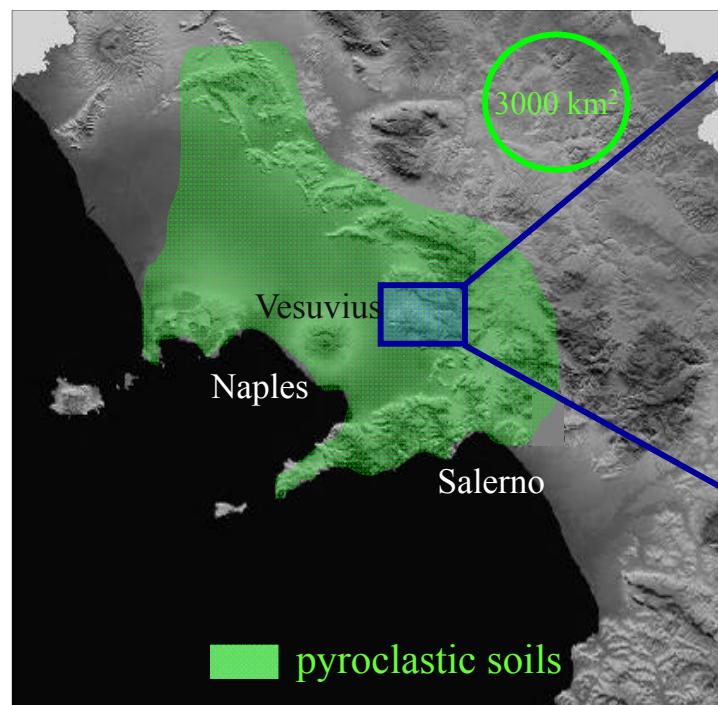
} bed entrainment

} huge final volume  
( $V_f = 20,000 \text{ m}^3$ )

$$\underline{\underline{A_f = 57.9}}$$

## MORE EXAMPLES

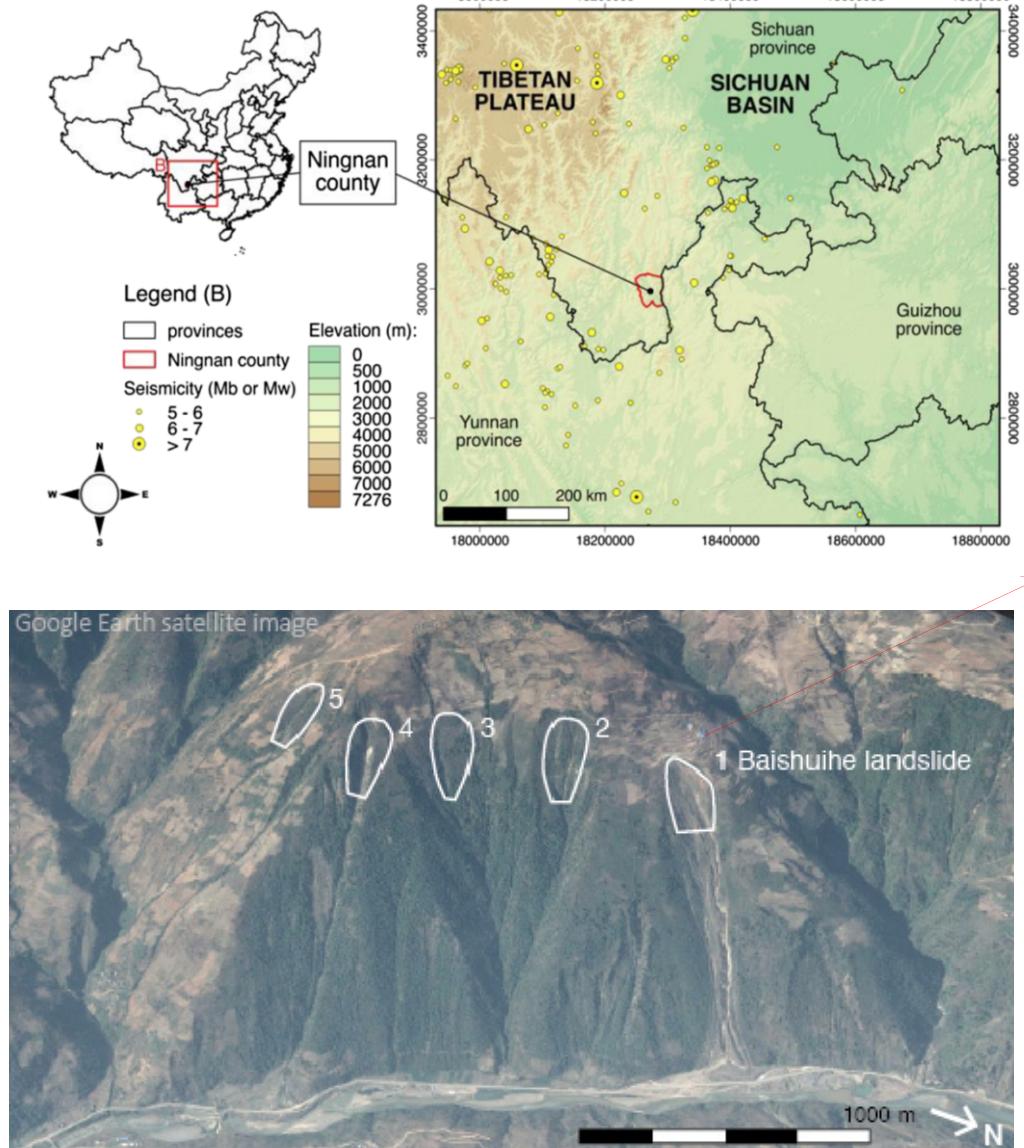
Study area (Vesuvius district)



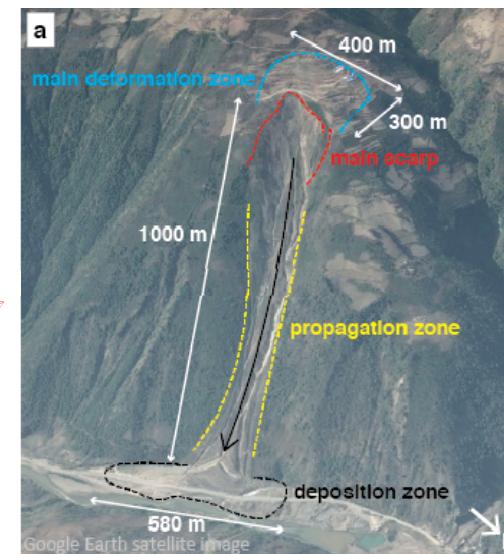
<http://www.commissario2994.it>

Cascini L., Cuomo S., Guida D. (2008). Typical source areas of May 1998 flow-like mass movements in the Campania region, Southern Italy. *Engineering Geology*, 96, pp. 107 - 125 (doi:10.1016/j.enggeo.2007.10.003)

## AND MORE EXAMPLES ...



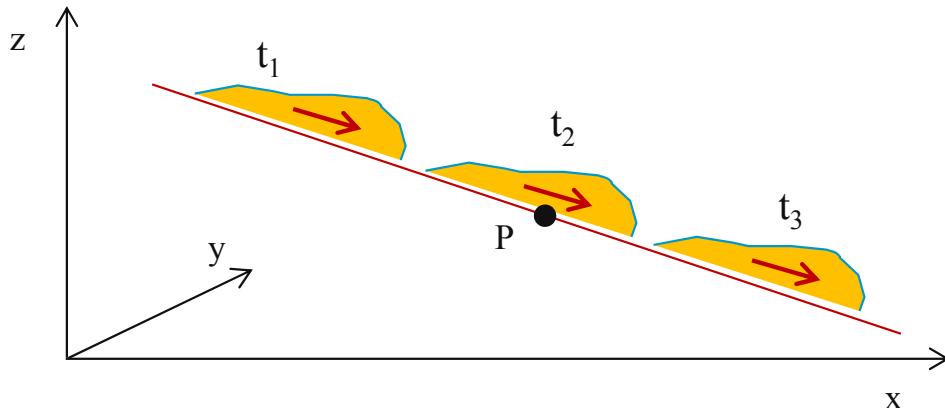
Sichuan province (China)



Braun, A., Wang, X., Petrosino, S., Cuomo, S. (2017). SPH propagation back-analysis of Baishuihe landslide in south-western China. *Geoenvironmental Disasters*, 4(1), 2.

Braun, A., Cuomo, S., Petrosino, S., Wang, X., L. Zhang (2017). Numerical SPH analysis of debris flow run-out and related river damming scenarios for a local case study in SW China. *Landslides*. (DOI: DOI<sup>https://doi.org/10.1007/s10346-017-0885-9</sup>)

## SCHEME



$$e_r = -\partial z / \partial t \quad e_r: \text{entrainment (or erosion) rate}$$

$$h_{er} = \int_0^{t_{er}} e_r dt \quad h_{er}: \text{erosion height @ point P (during } t_{er})$$

$$V_{er} = \int_{v,t} h_{er} dv dt \quad V_{er}: \text{volume eroded (in whole landslide)}$$

$$V_f = V_i + V_{er} \quad V_f, V_i: \text{final (and initial) volume}$$

$$A_f = V_f / V_i \quad A_f: \text{amplification factor}$$

## FACTORS

- propagation height:  $h(x, y, z, t)$  ? yes
- velocity:  $v(x, y, z, t)$  ? yes
- ground slope angle:  $\theta(x, y, z, t)$  ? yes

Hungr et al. (2005)

$$e_r = h \cdot v \cdot E_S$$

$$E_S = \ln \left( \frac{V_f}{V_i} \right) \frac{1}{L} \quad E_s: \text{from empirical observations}$$

Blanc et al. (2011)

$$e_r = h \cdot v \cdot \tan(\vartheta)^{2.5} \cdot K \quad K: \text{empirical factor}$$

and many other contributions .....  
(a review in Pirulli and Pastor, 2012)

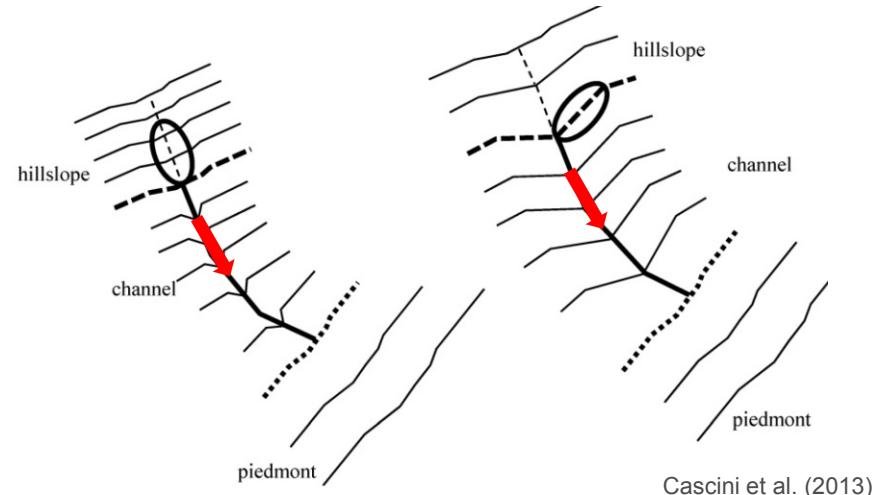
Hungr, O., Corominas, J., Eberhardt, E., 2005. Estimating landslide motion mechanism, travel distance and velocity. *Landslide Risk Management*. 99-128.

Blanc, T., Pastor, M., Dremptic, M. S. V., Haddad, B. 2011. Depth integrated modelling of fast landslide propagation. *European Journal of Environmental and Civil Engineering*, 15(sup1), 51-72.

Pirulli, M., Pastor, M. (2012). *Geotechnique* 62, No. 11, 959–972.

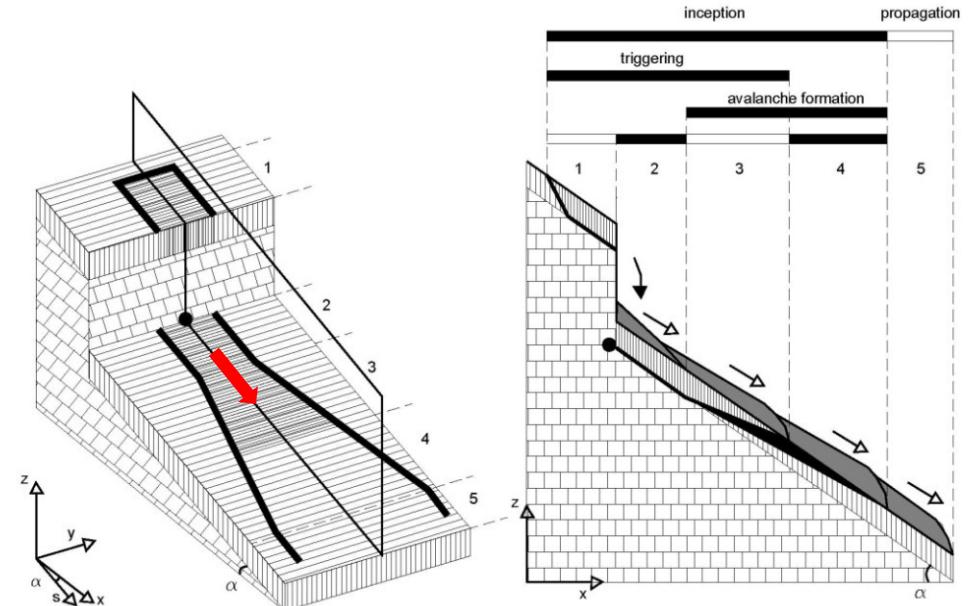
## DEBRIS FLOWS

- “V” shaped channels (with steep flank)
- entry of channel, height and velocity increase
- along the channel, **bed entrainment**
- exit of channel, stopping/diversion/bifurcations
- shape/extent of propagation area **unknown a priori**



## DEBRIS AVALANCHES

- open slopes (constant soil cover and slope angle)
- zone 1, small slides upslope bedrock outcrops
- zone 2, impact / water springs below bedrock outcrops
- zone 3, trust of failed material and/or **bed entrainment**
- zone 4, **bed entrainment**
- width of zones 3 e 4 **unknown a priori**



Cascini L., Cuomo S., Pastor M. (2013). Inception of debris avalanches: Remarks on geomechanical modelling. *Landslides*, 10(6), 701-711.

Cascini L., Cuomo S., Pastor M., Sorbino G., Piciullo L. (2014). SPH run-out modelling of channelized landslides of the flow type. *Geomorphology*, 214, 502-513.

## AND SO?

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Better, go simple (for the moment) !

Why?

- Relevant Factors are many.
- Soil is never bare (vegetation, trees, even waste material along slopes)
- And, what about real field measurement of bed entrainment?  
as most, order of magnitude or eroded heights, few (or some) local measurements

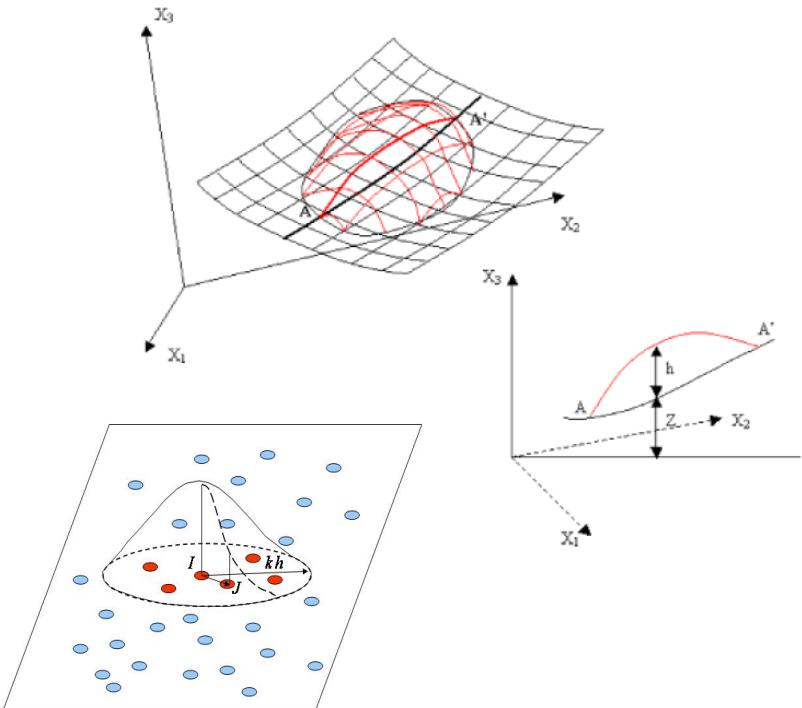
But, entrainment is a  $f(x, y, z, t)$

Yes, go simple with Hungr et al. (2005) and Blanc et al. (2011) EMPIRICAL models

# GeoFlow-SPH depth-integrated coupled model

➤ Proposed by Pastor et al. (2009)

➤ equations:



balance of mass, depth integrated equation:

$$\frac{Dh}{Dt} + h \operatorname{div}(\bar{v}) = e_r$$

balance of linear momentum, depth integrated equation :

$$\rho \frac{D(h\bar{v})}{Dt} + \operatorname{grad}\left(\frac{1}{2} \rho g h^2\right) = -\frac{1}{\rho} e_r \bar{v} + \rho b h + \operatorname{div}(h\bar{s}) - \rho g h \operatorname{grad} Z - \underline{\tau_b} - \rho h \bar{v} \operatorname{div}(\bar{v})$$

defined by frictional rheological model:

$$\underline{\tau_b} = -(1-n)(\rho_s - \rho_w)g \cdot h - p_w^b \tan \phi_b \cdot \operatorname{sgn}(\bar{v})$$

$\tau_b$  = shear stress at the basal surface

$p_w$  = pore pressure at the basal surface

1D vertical consolidation depth integrated equation:

$$\frac{\partial}{\partial t} \left( \underline{\underline{P}}_{w_1} h \right) + \frac{\partial}{\partial x_k} \left( \bar{v}_k \underline{\underline{P}}_{w_1} h \right) = -\frac{\pi^2}{4h^2} c_v \underline{\underline{P}}_{w_1}$$

entrainment law (Hung et al., 2005):

$$\underline{\underline{e}_r} = \underline{\underline{E}_s} \times h \times \bar{v}$$

$h$  = flow depth

$v$  = depth-averaged velocity

$E_s$  (amplification rate) is independent on flow depth and velocity

$V_i$  = volume entering the erosion zone

$V_f$  = total volume exiting the erosion zone,

$l$  = average path length of the erosion zone

$$\underline{\underline{E}_s} = \ln \left( \frac{V_f}{V_i} \right)^{\frac{1}{l}}$$

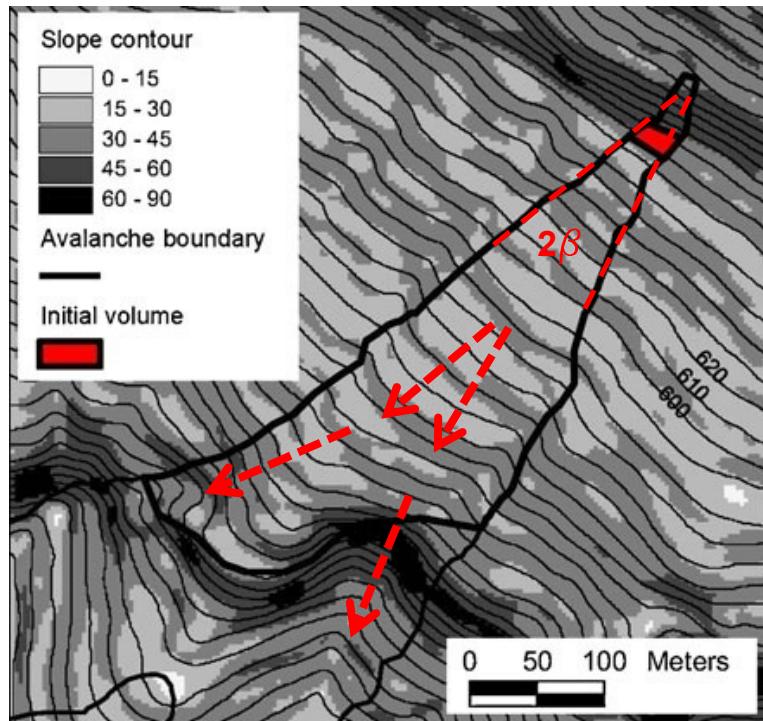
entrainment law (Blanc et al., 2011):

$$\underline{\underline{e}_r} = K \cdot v \cdot h \cdot (\tan \theta)^{2.5}$$

$K$  = empirical factor

$\theta$  = slope angle of ground surface

## CONSEQUENCES



- hillslope 35° steep
- $B_{\text{trig}}=30\text{m}$ ,  $L_{\text{trig}}=25\text{m}$ , initial mass **1'400 m<sup>3</sup>**
- mobilized volume **30'000 m<sup>3</sup>** ( $A_f = 75$ )
- 510 m propagation along an open slope
- lateral spreading ( $2\beta=20^\circ$ )
- bifurcation of the mass into two debris flows

Cortadonica Debris Avalanche (Italy), 1998

## NUMERICAL MODEL

- Digital Elevation Model (DTM) 3m x 3m
- 639 points (initially 1 m spaced)
- Runge-Kutta numerical algorithm( $dt \leq 0.1$  s)

## ANALYZED CASES

- friction angle ( $\phi'$ ):  $22.5^\circ$ ,
- water table height  $\approx 1/4$  propagation height ( $h_w^{\text{rel}}=0.25\div 0.4$ )
- pore water pressures from literature ( $p_w^{\text{rel}}=1$ )
- entrainment :  $E_s = 4.0 \times 10^{-3} \text{ m}^{-1}$

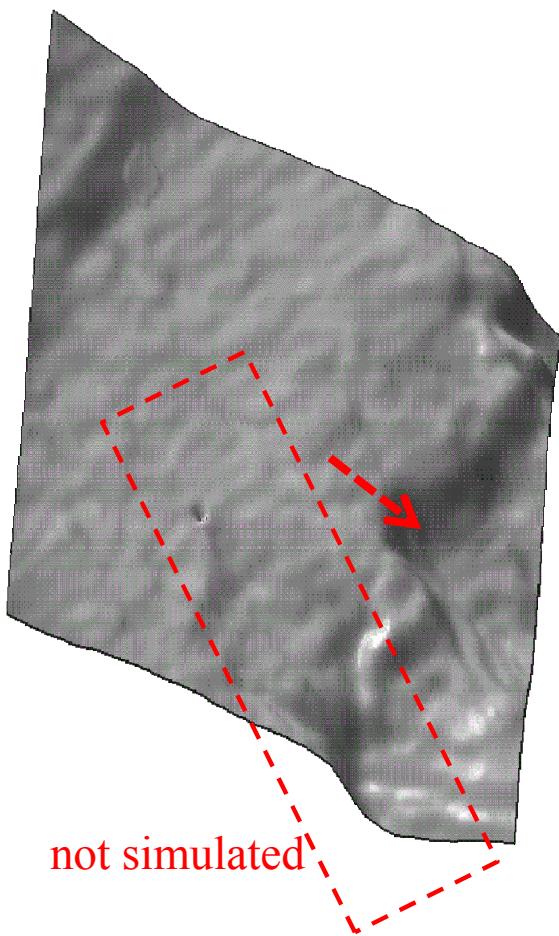
## Case #1 – SPREAD & SPLIT

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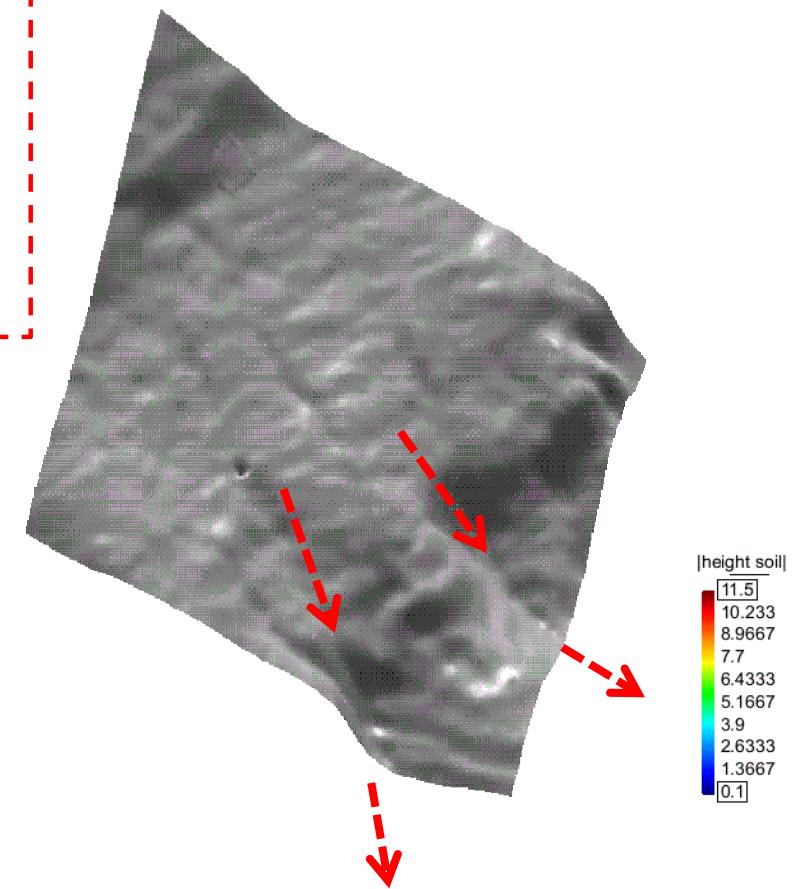
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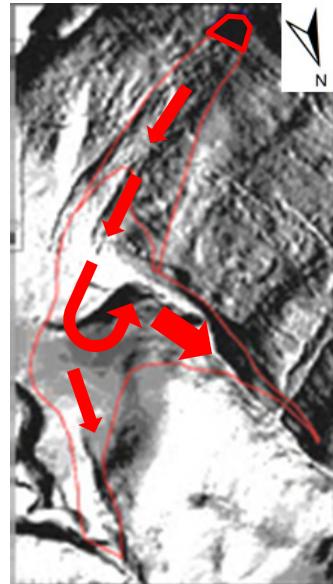
Field evidence

$E_s = 0$



Landslide amplification rate:  $E_s = 4.0 \times 10^{-3}$





Cuomo et al. (2014)

- mobilized volume 30'000 m<sup>3</sup>
- 360 m propagation along an open slope ( $2\beta=14^\circ$ )
- run-up on the opposite slope  $\approx 10$  m
- (partial) deposition at the base of the slope
- bifurcation of the mass into two debris flows
- main debris flow propagated 1'400 m

### Cervinara Debris Avalanche (Italy), 1999

#### NUMERICAL MODEL

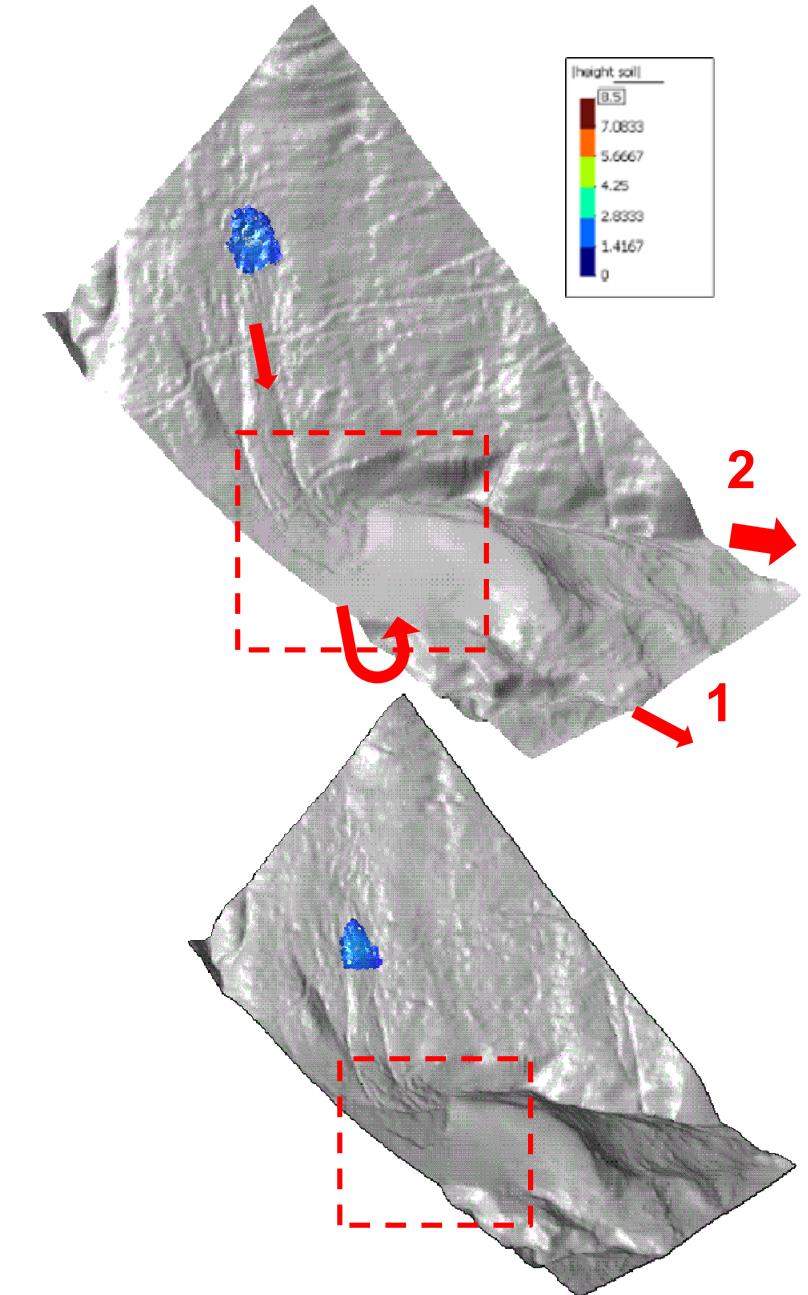
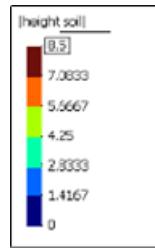
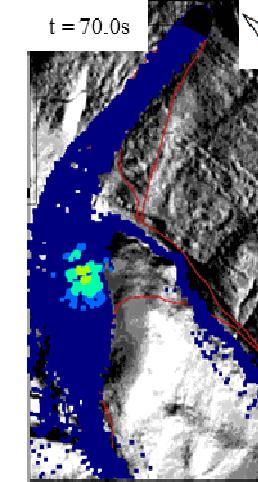
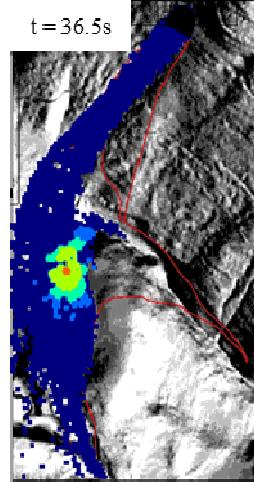
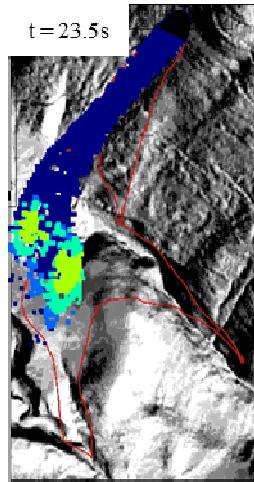
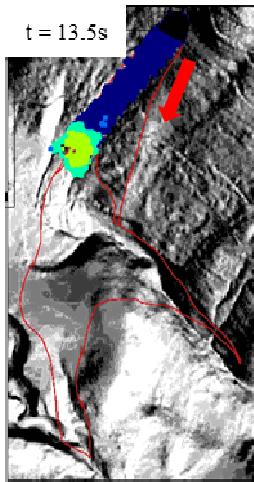
- Digital Elevation Model (DTM) 2m x 2m
- 1'600 points (initially 1 m spaced)
- Runge-Kutta numerical algorithm( $dt \leq 0.8s$ )

#### ANALYZED CASES

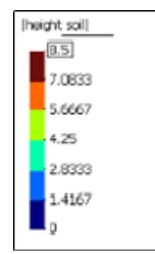
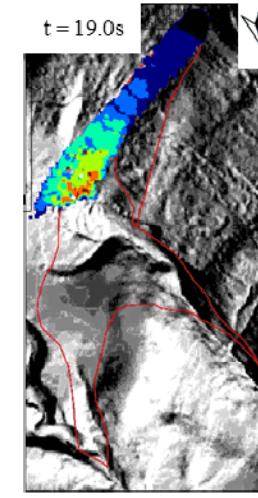
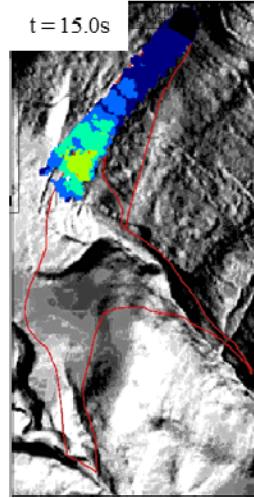
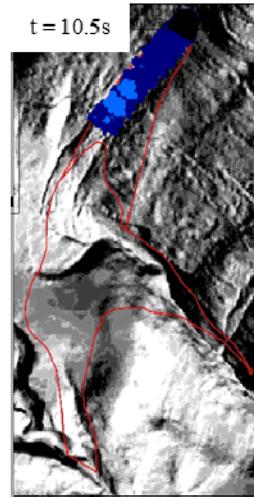
- friction angle ( $\phi'$ ):  $9^\circ \div 24^\circ$ ,
- water table height  $\approx 1/2$  propagation height ( $h_w^{\text{rel}}=0.5 \div 1$ )
- pore water pressures from literature ( $p_w^{\text{rel}}=1$ )
- distinct hypotheses for entrainment:  $E_s = 2 \times 10^{-3} \div 10^{-2} \text{ m}^{-1}$

## Case #2 – SPREAD, PROPAGATION, DEPOSITION AND SPLIT

Landslide amplification rate ( $E_S$ ):  $1.0 \times 10^{-2} \text{ m}^{-1}$

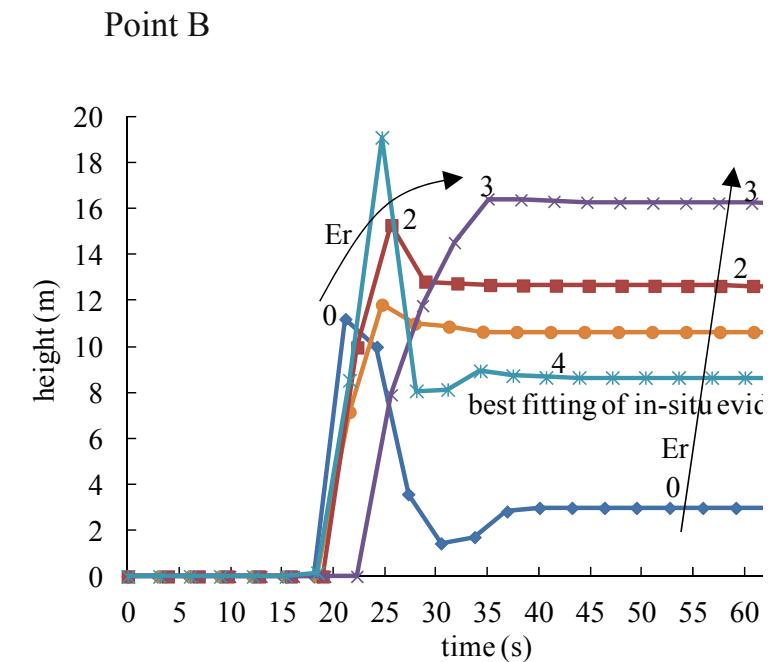
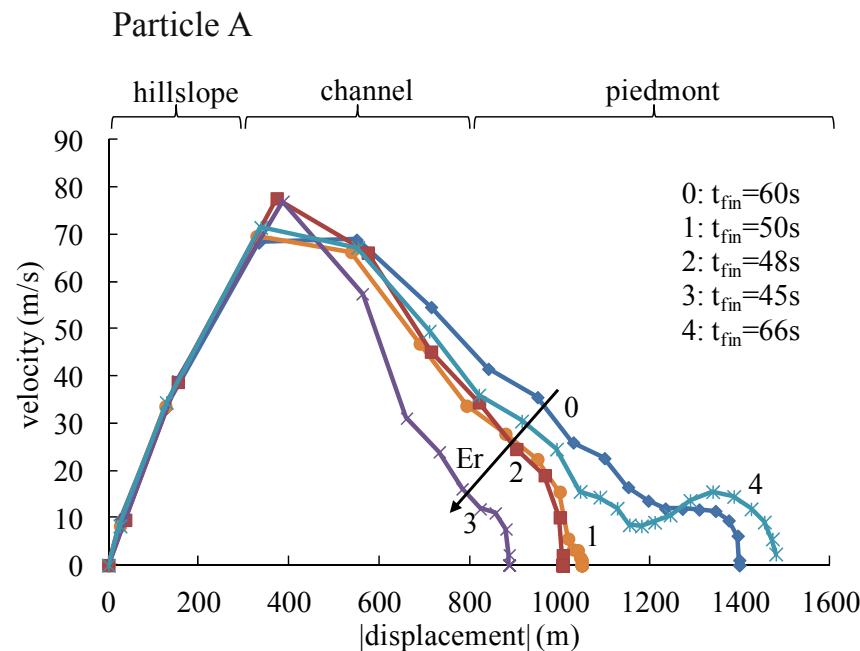
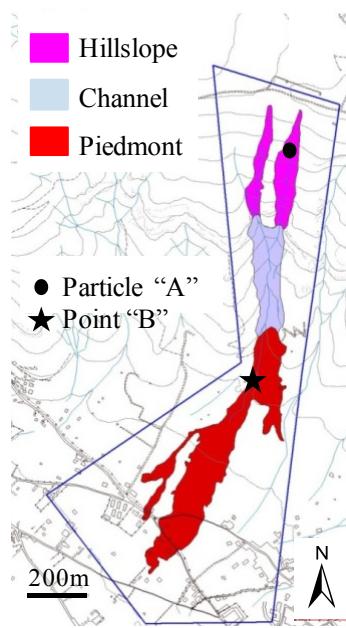


$E_S = 0$



## Case #3 – DEPOSITION

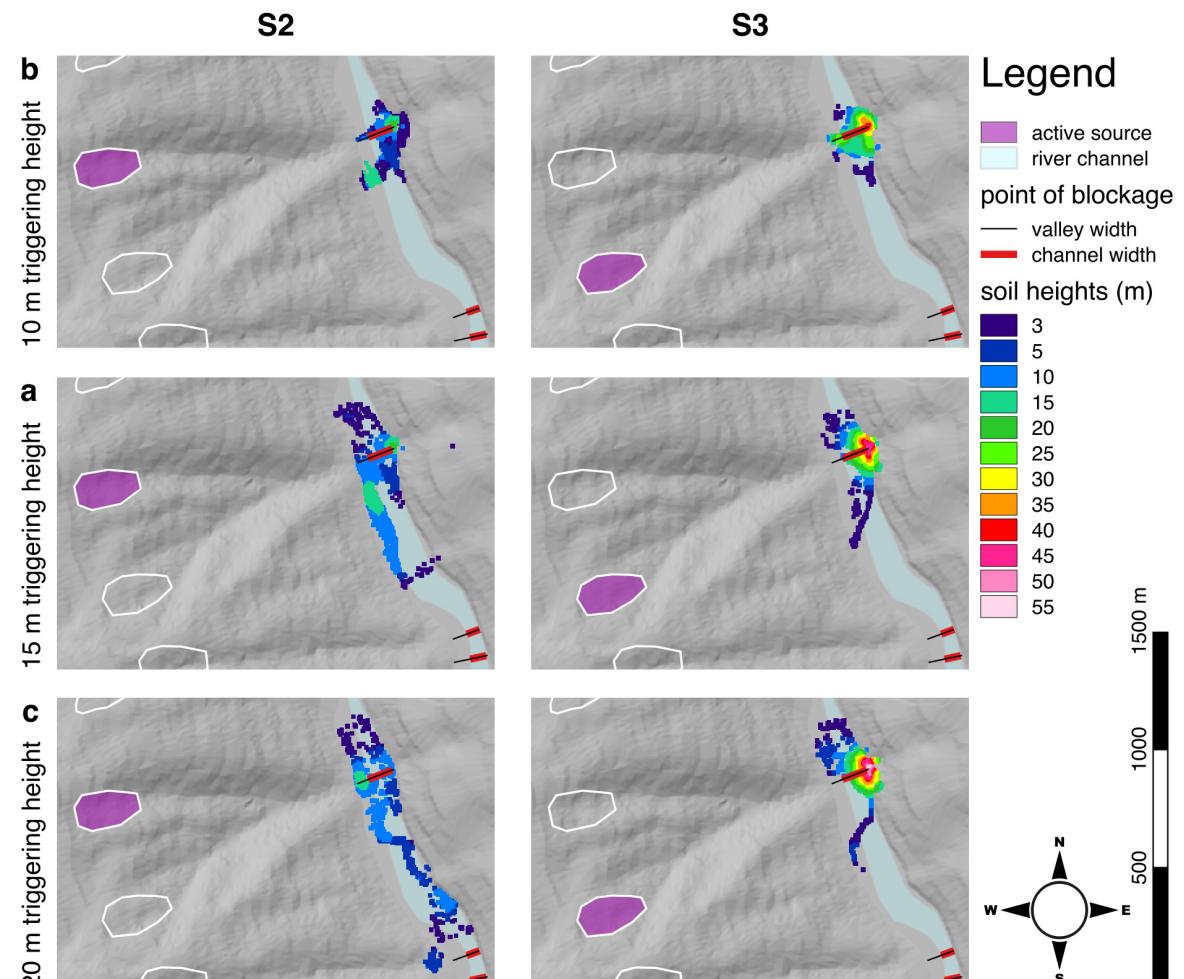
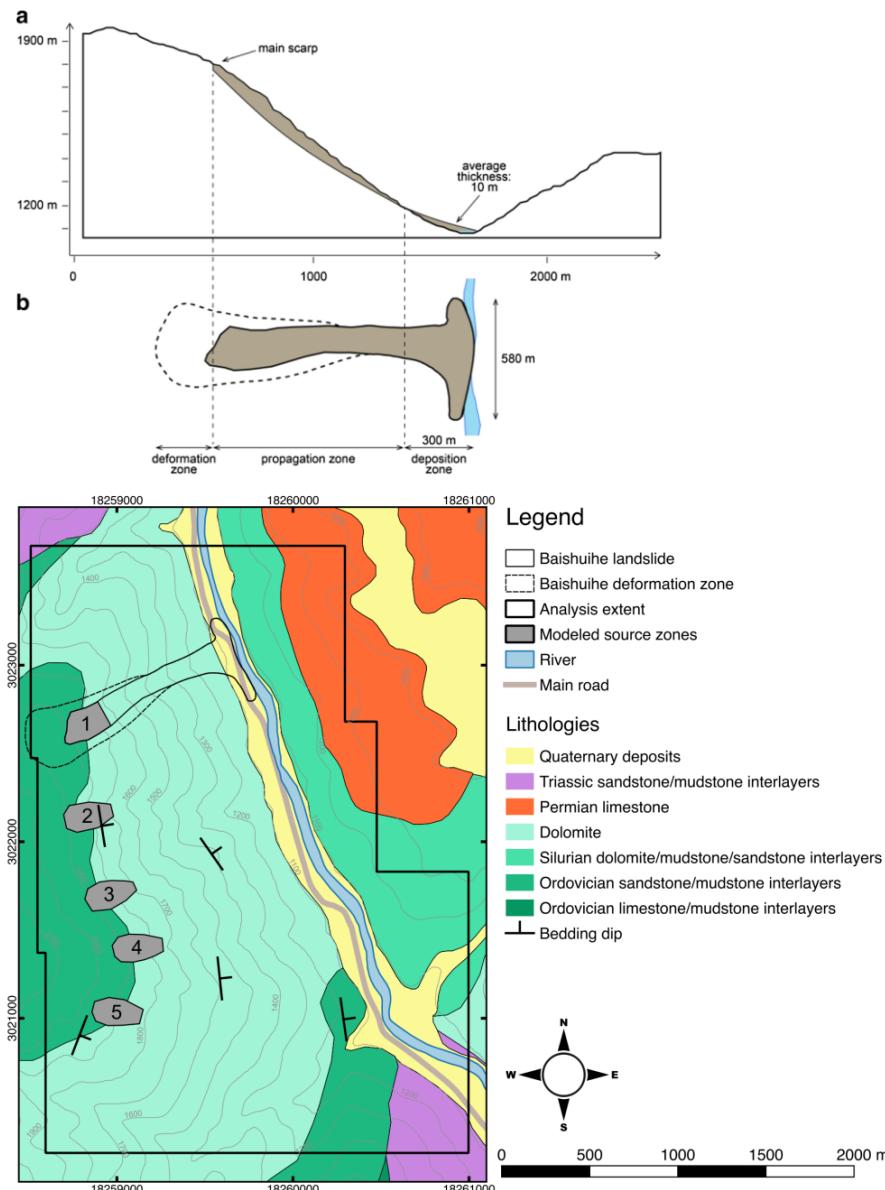
Tuostolo Debris Flow (Italy), 1998



- erosion reduces landslide velocity at the front (so, in the channel and piedmont area)

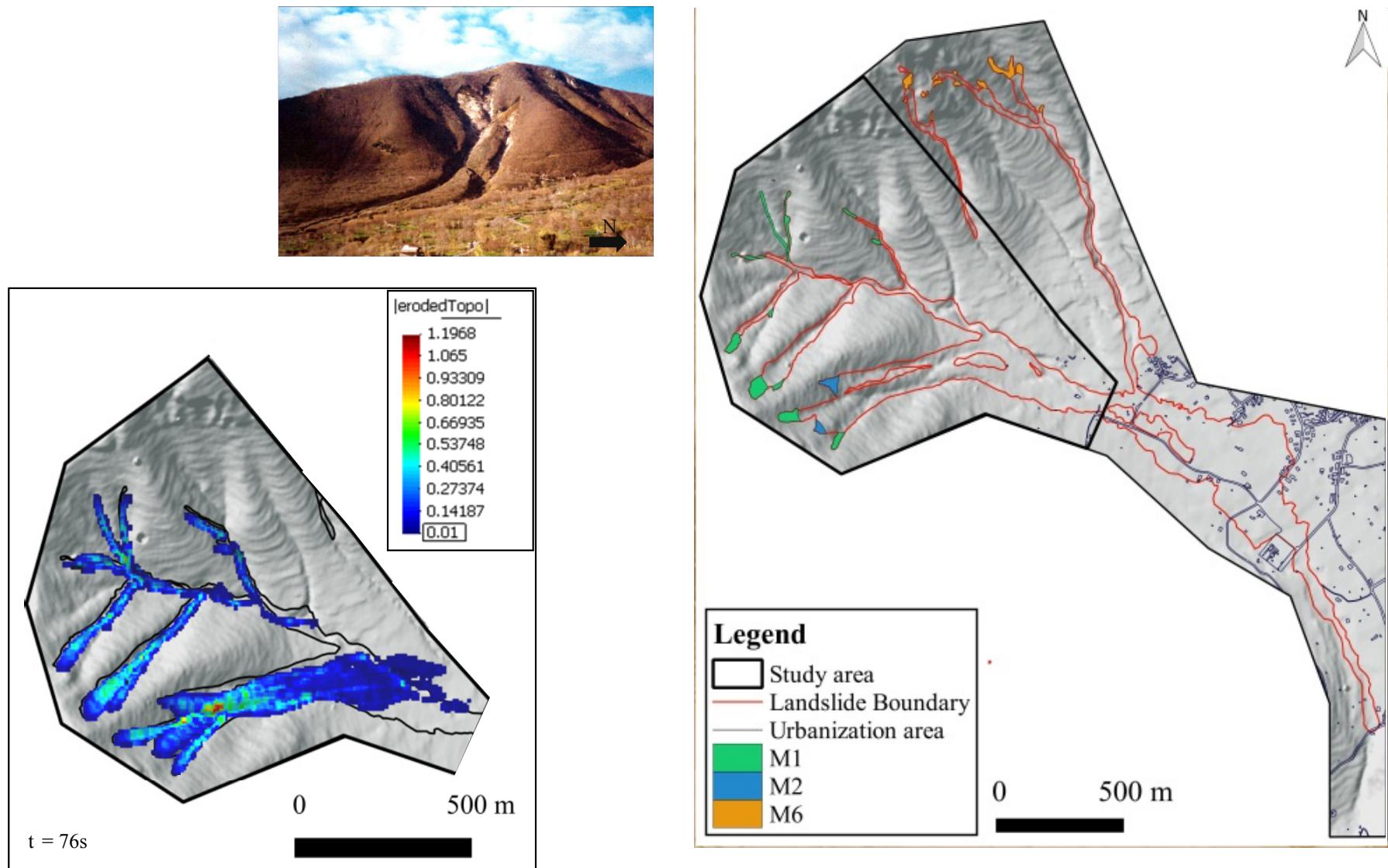
## Case #4 – LANDSLIDE DAM

### Baishuihe Debris Flow (Sichuan, China), 2012



## Case #5 – MULTIPLE MIXED FLOWS

Combination of Debris Flow and Debris Avalanche (Italy), 1998



## COUNTEMEASURES

## Examples of control works

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Control of surface erosion

Installation of biomats



Reinforced terrains

Soil nailing



Obstacles to the flow

Disposition of baffles



Installation of geotexiles



Reinforced soils

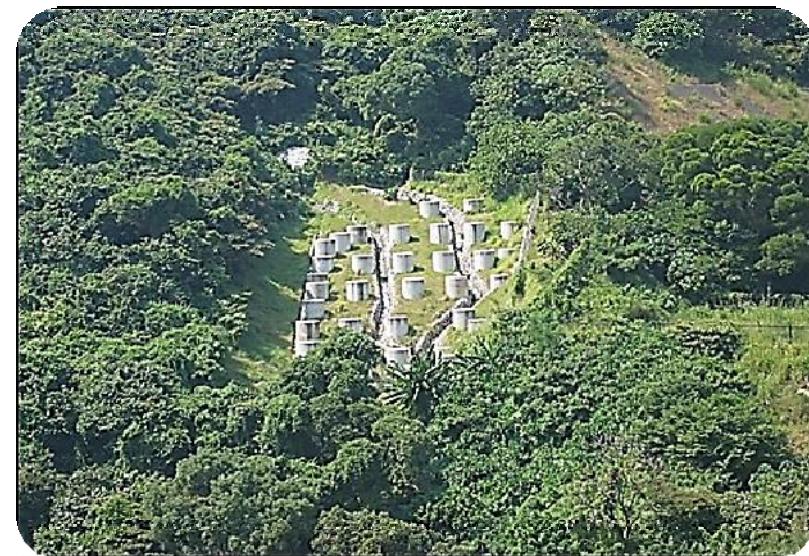
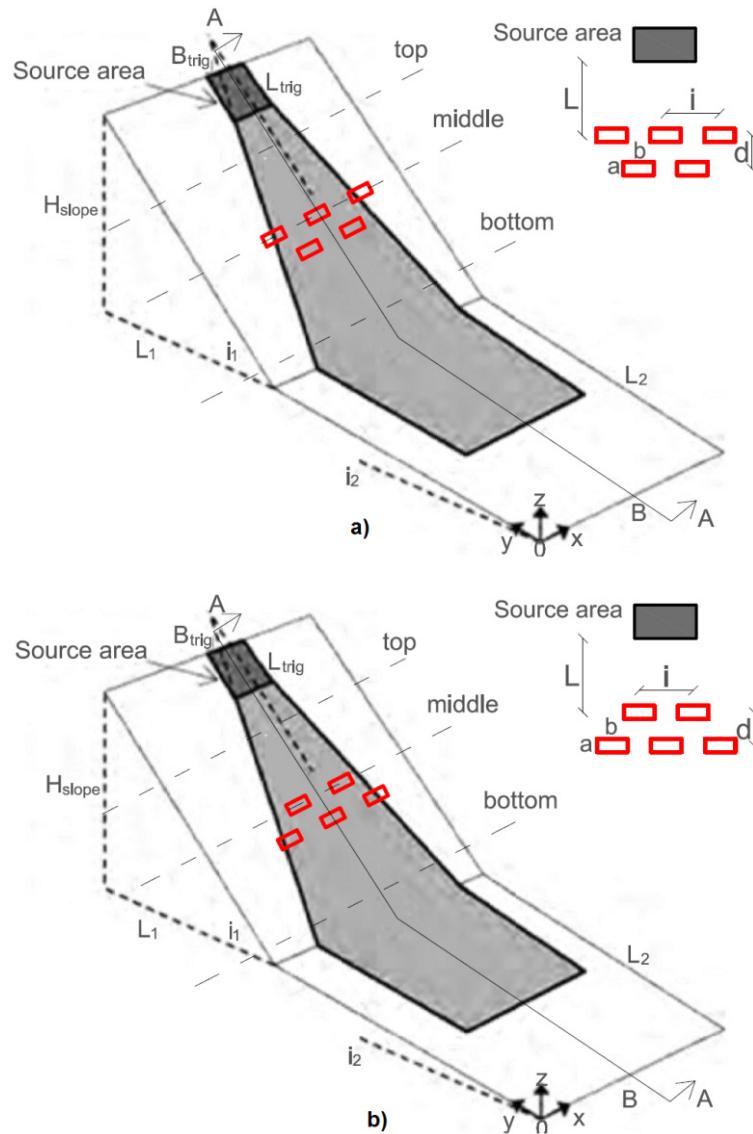


Disposition of barriers



# SLOPE ENGINEERED

just  $3 \times 2$  combinations of 5 baffles



Case (-)	a (m)	b (m)	i (m)	d (m)	L (m)
3+2 top	10	5	30	15	30
3+2 middle	10	5	30	15	80
3+2 bottom	10	5	30	15	130
2+3 top	10	5	30	15	30

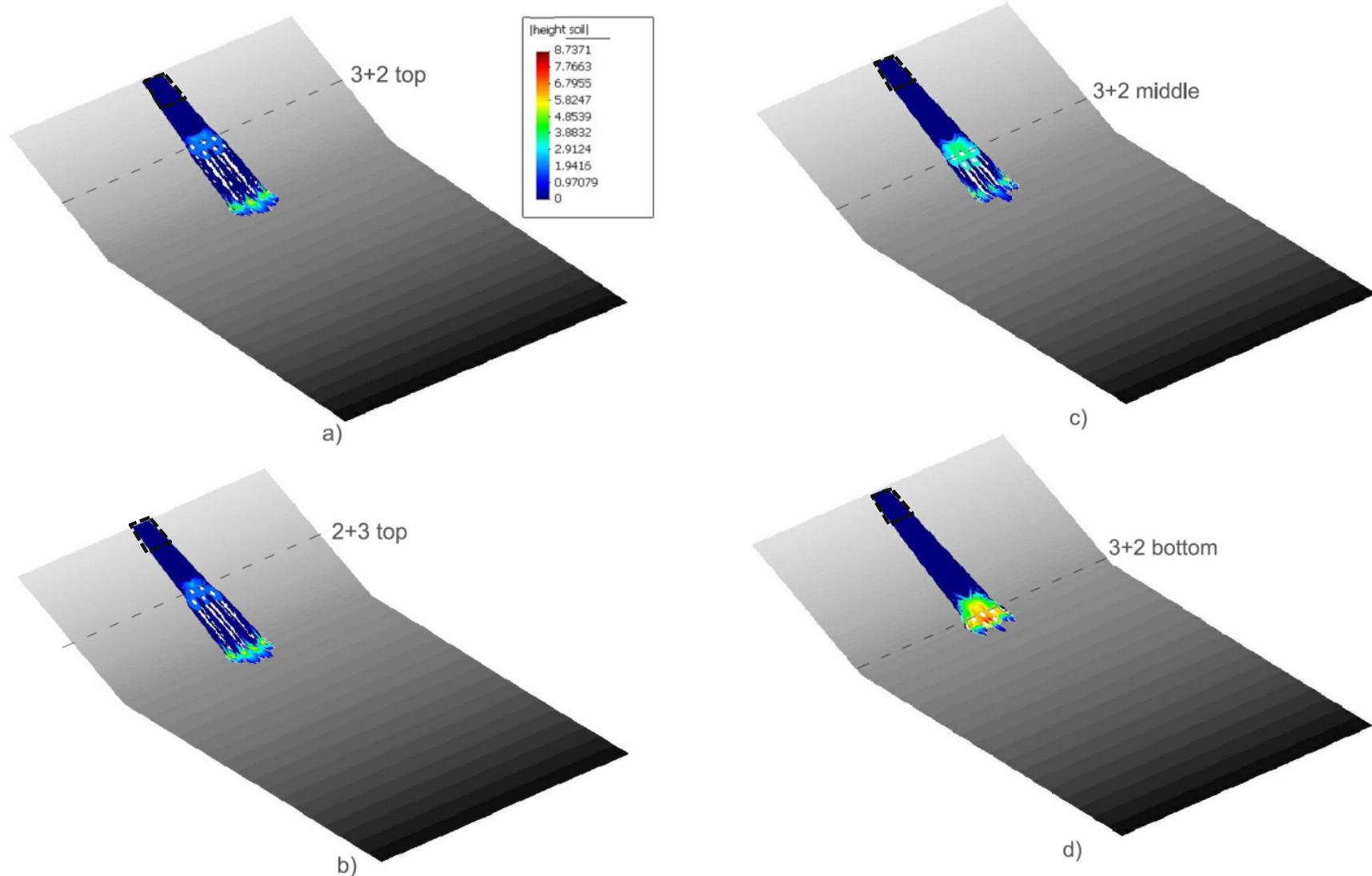
$\tan\phi_b$ (-)	$h_{rw}^{rel}$ (-)	$p_{rw}^{rel}$ (-)	$B_{fact}$ ( $m^2 s^{-1}$ )	$K_r$ (-)
0.50	0.4	0.5	$1.0 \times 10^{-2}$	$3.0 \times 10^{-2}$

# SLOPE ENGINEERED with BAFFLES

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## Thickness of Landslide Deposit



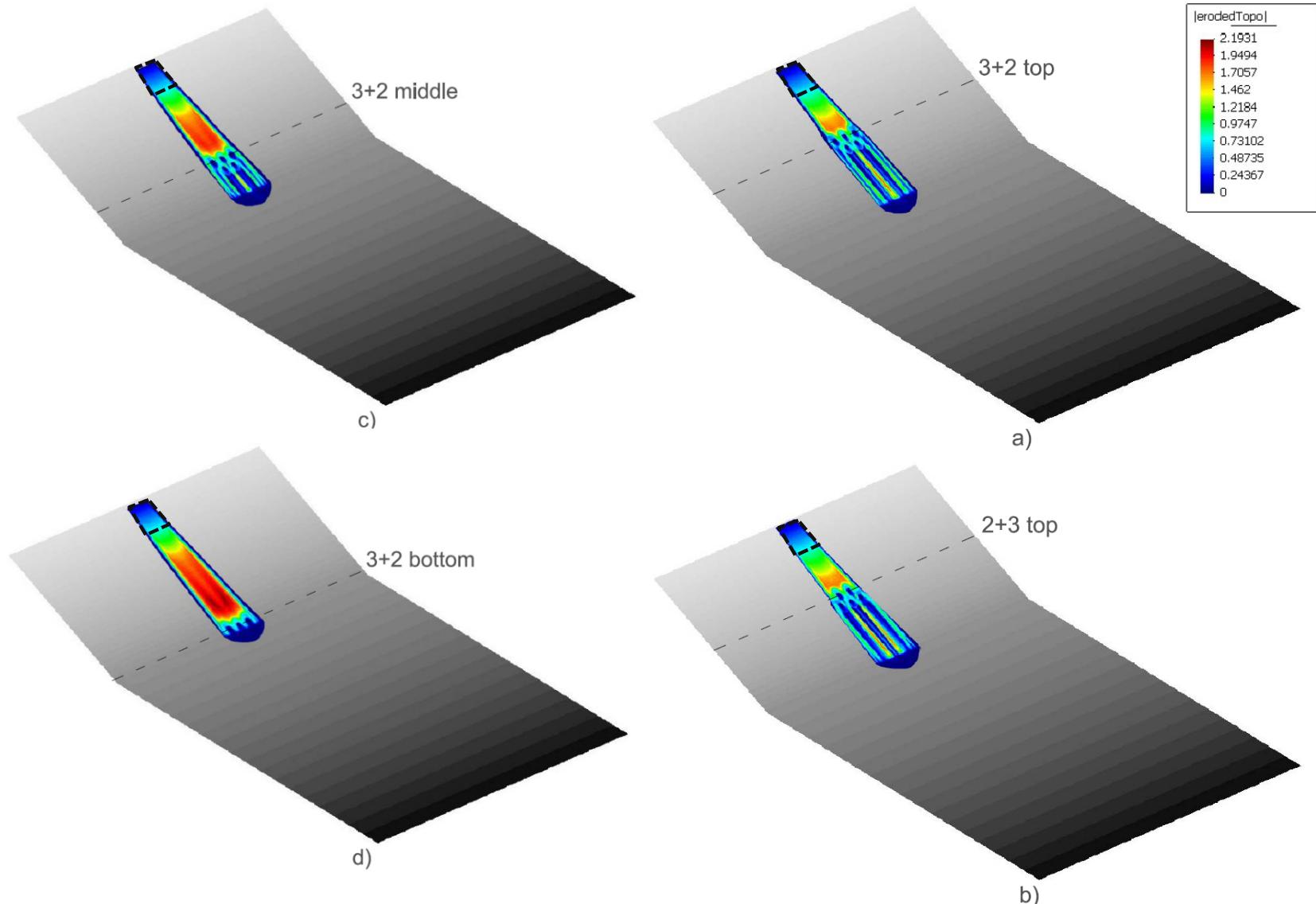
Cuomo S., Cascini L., Pastor M., Petrosino S. (2017). Modelling the propagation of debris avalanches in presence of obstacles. In: 4th World Landslide Forum 2017. Springer International Publishing AG 2017, M. Mikos et al. (eds.), Advancing Culture of Living with Landslides, DOI 10.1007/978-3-319-53487-9\_55

# SLOPE ENGINEERED with BAFFLES

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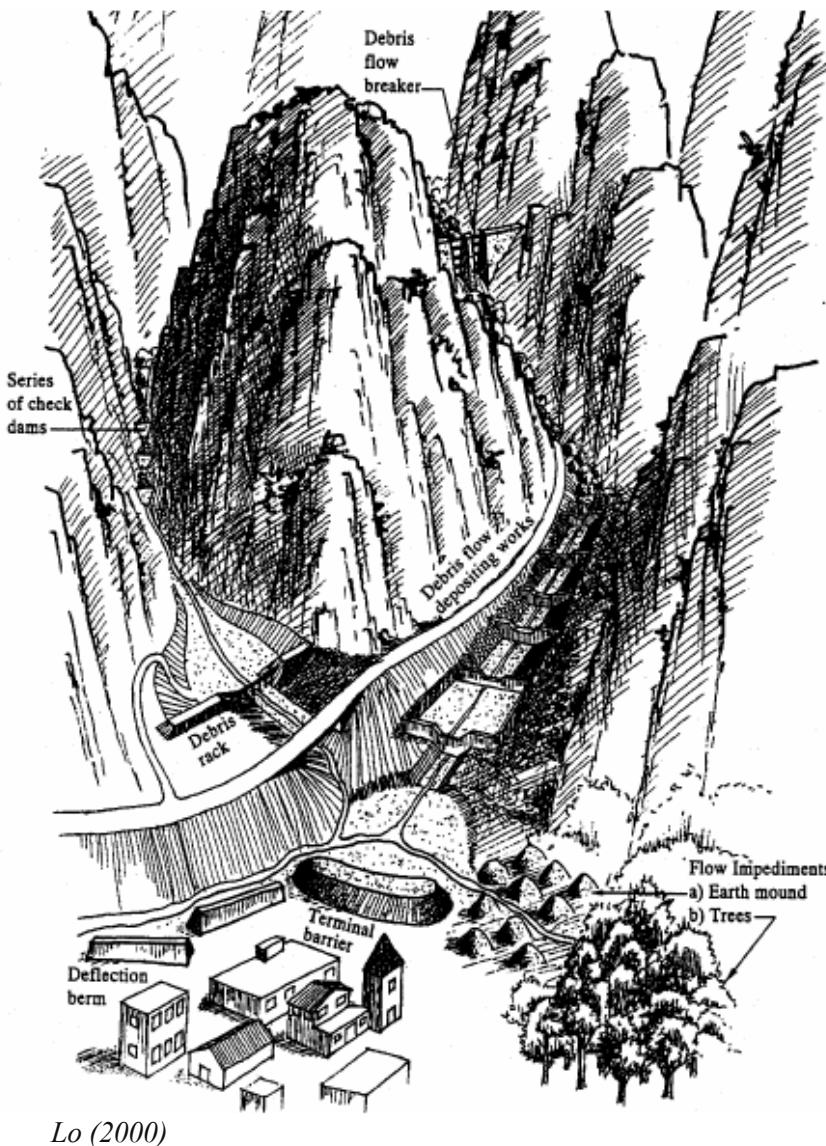
Soil eroded along the slope



Cuomo S., Cascini L., Pastor M., Petrosino S. (2017). Modelling the propagation of debris avalanches in presence of obstacles. In: 4th World Landslide Forum 2017. Springer International Publishing AG 2017, M. Mikos et al. (eds.), Advancing Culture of Living with Landslides, DOI 10.1007/978-3-319-53487-9\_55

## Remarks and Conclusions

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Lo (2000)

- ✓ Mechanisms: relatively clear
- ✓ Modelling of entrainment: empirical
- ✓ Data from the field: **problematic!!**
- ✓ Existing analytical models cannot be used in real cases

So, under **simple hypotheses**,

- ✓ modelling of the consequences: yes

**Contributions of geomechanics:**

- ✓ understand where and which field measurements could be useful,
- ✓ develop analytical models (effectively usable with the data available)
- ✓ implementation of such new analytical models in the already existing powerful computation tools

# Thank you for your attention

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