



# COUPLING BETWEEN MECHANICAL STATE AND PERMEABILITY FOR CONCRETE

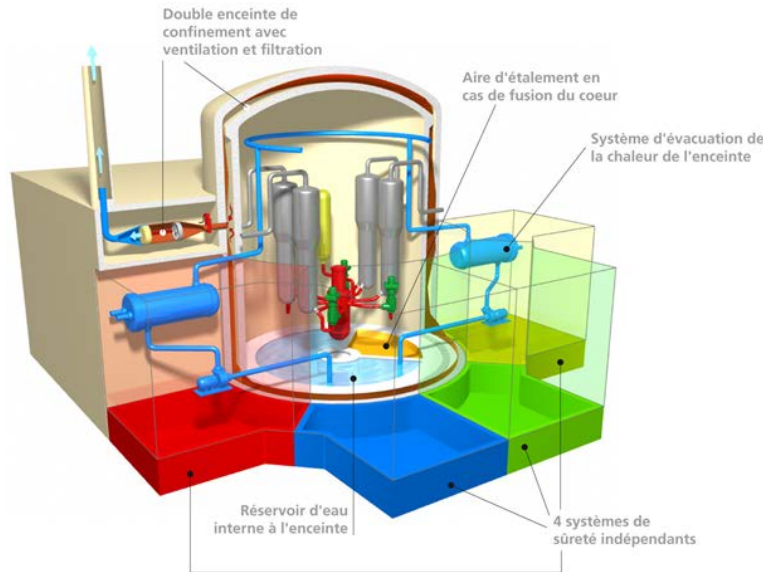
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France*



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## Introduction & context

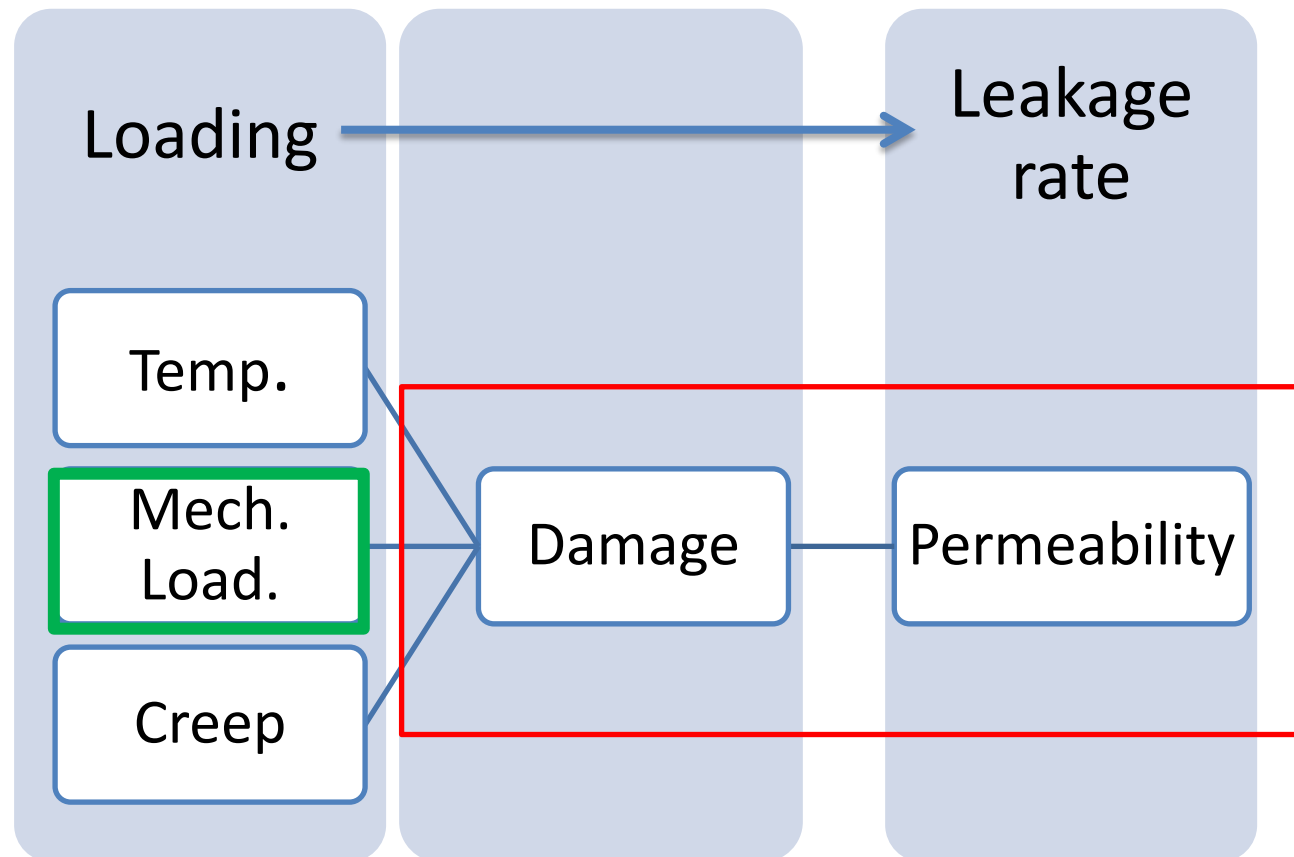


- Sealing has to be guaranteed for 40 years.
- **Diffuse damage (microcracking) and/or localized (macrocracking)**
- **Estimation of the evolution of transfer properties (structural durability analysis)**

### Objectif:

**Propose/validate numerical tools to predict/estimate leakage rate in a cracked structure.**

## Numerical modelling



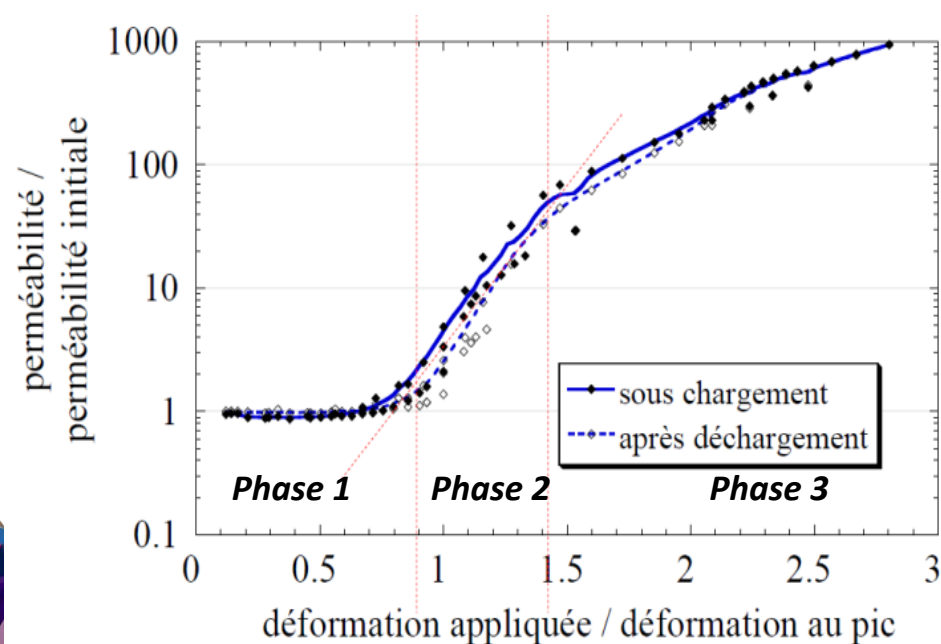
Here we focus on studying the effect of mechanical damage on the permeability of concrete.

# Outline

- ▶ - **Bibliography**
- ▶ - Hydro-mechanical modelling
- ▶ - Application: Brazilian test
- ▶ - Conclusions
- ▶ - Perspectives

## Effect of the mechanical load on the permeability of concrete

[Hearn 1999] [Aldea 2000] [Picandet 2001, 2009] [Biparva 2005]  
[Choinska 2007] [Dal Pont 2011 (HDR)] [Rastiello 2014]



Permeability evolution of concrete during compression  
test [Choinska et al. 2007]

- Difficulty to perform the coupling during the three phases with the same model

### Phase 1

- Low evolution of sample permeability
- Permeability governed by Darcy's law

$$k_m = \mu \frac{Q}{A} \left( \frac{\Delta P}{\Delta x} \right)^{-1}$$

### Phase 2

- high permeability evolution
- Crack connectivity

### Phase 3

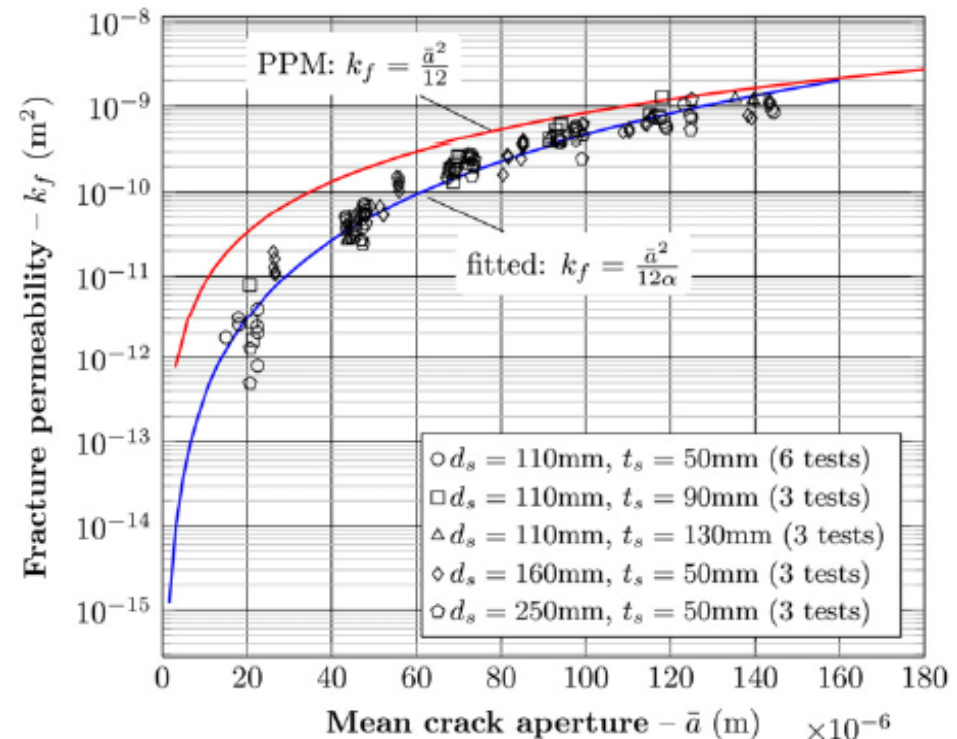
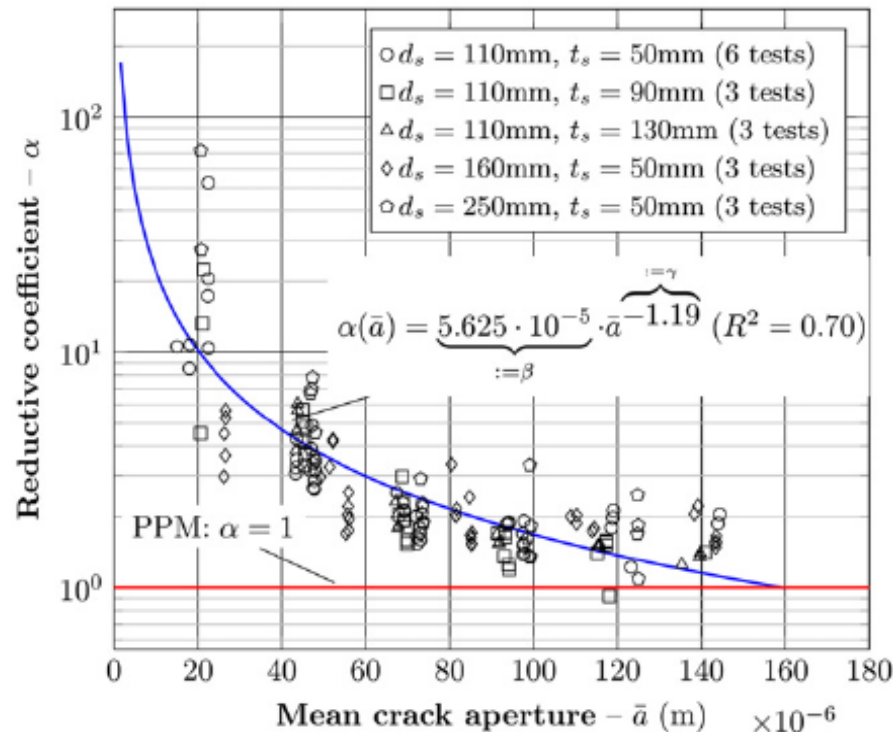
- Permeability increased by 3 orders of magnitude
- Governed by Poiseuille's law

$$k_P = \frac{[u]^2}{12\alpha}$$

$\alpha$  : crack roughness, tortuosity and bridging

## Discussion on the correction factor $\alpha$

[Hearn 1999] [Aldea 2000] [Picandet 2001, 2009] [Biparva 2005]  
[Choinska 2007] [Dal Pont 2011 (HDR)] [Rastiello 2014]



a) Correction factor versus mean crack aperture

[Rastiello et al. 2014]

- Correction factor is adopted in the numerical simulations

b) fracture permeability estimation via the experimentally corrected parallel plates model

## Continuous modelling: damage model

### Isotropic damage model

#### Local model

$$\varepsilon_{eq} = \sqrt{\sum_{i=1}^3 \langle \varepsilon_i \rangle_+} \quad \text{Mazars 1986}$$

- Can be applied at the scale of the structure.
- Simple, time cost is reasonable and accurate mechanical description.
- Crack opening can be calculated.
- No need before mechanical description information about the crack.

#### Nonlocal integral model

Pijaudier-Cabot and Bažant 1987

$$\phi_0(\mathbf{x}, \mathbf{s}) = \exp \left( - \left( \frac{4 \|\mathbf{x} - \mathbf{s}\|^2}{l_c^2} \right) \right)$$

#### Stress based nonlocal model

Giry et al. 2011

$$\phi_0(\mathbf{x}, \mathbf{s}) = \exp \left( - \left( \frac{4 \|\mathbf{x} - \mathbf{s}\|^2}{l_c^2(\mathbf{x}, \boldsymbol{\sigma}_{prin}(\mathbf{s}))} \right) \right)$$

$$\overline{\varepsilon_{eq}}(\mathbf{x}) = \frac{\int_{\Omega} \phi_0(\mathbf{x}, \mathbf{s}) \cdot \varepsilon_{eq}(\mathbf{s}) d\mathbf{s}}{\int_{\Omega} \phi_0(\mathbf{x}, \mathbf{s}) d\mathbf{s}}$$



## ***Coupling between permeability and mechanical state (Continuous approach)*** [Pijaudier-Cabot et al, JEM (2009)]

(For each element i)

$$\log K_i = D \log K_P + (1 - D) \log K_D$$

Poiseuille's Permeability:  
(Mean Permeability of a cracked element)

$$K_P = \xi \frac{(\lambda l_c)^3}{12 l_e} (F^{-1}(D) - Y_{D0})^3$$

$$F^{-1}(D) = Y_{D0} - \frac{\ln(1 - D)}{B_t}$$

$\xi$  : correction factor for Poiseuille's Law  
(Roughness, Tortuosity, etc..)

Picandet's Permeability:  
(Permeability of a microcracked element)

$$K_D = K_0 f(D) = K_0 \exp \left( (\alpha D)^\beta \right)$$

$K_0$  : Permeability of a sound material

$\alpha, \beta$ : Fitted parameters

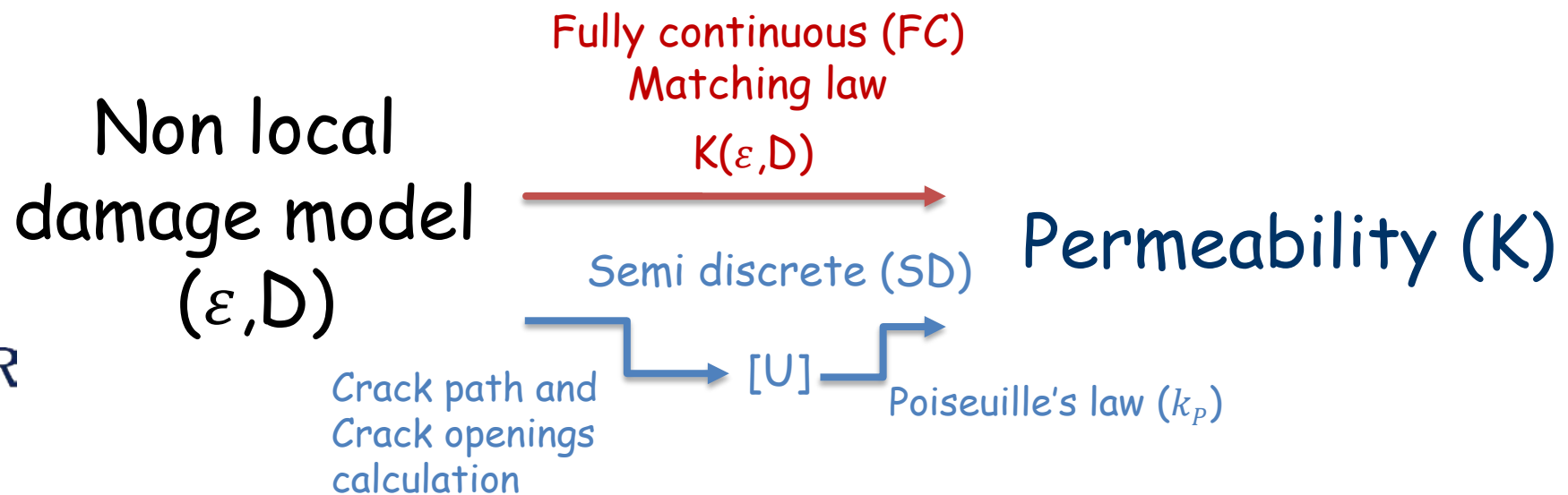
$D$  : Damage Field



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## *Coupling by means of two approaches*



- FC approach can be directly applied once the mechanical problem is solved.
- SD approach requires crack tracking and crack opening assessment

## Coupling by means of two approaches

### Fully continuous approach

### Semi discrete approach

#### Diffuse damage

$$k_D^e = k_0 f(D^e) = k_0 \exp((\alpha D^e)^\beta) \quad \text{Picandet et al. 2001}$$

$$D^e \leq l \quad (l = 0,15 \text{ for instance})$$

#### Localized damage (Poiseuille $k_P^e$ )

$$k_P^e = \frac{(l^e)^{2-\gamma_r}}{12\beta_r} (\varepsilon_{n^e} - \varepsilon_{D0})^{3-\gamma_r}$$

  $\varepsilon_{n^e}$  : Maximum principal strain

$\varepsilon_{D0}$  : Strain at first crack initiation

$l^e$  : Average length of the FE ( $\sqrt[3]{V^e}$ )

$$\xi = f([u], \beta_r, \gamma_r) \quad \text{Rastiello et al. 2014}$$

$$K_m^e = (k_D^e)^{1-D} \times (K_P^e)^D$$

$$\bullet \quad k_P^e = \frac{[u_{n^e}^e]^{3-\gamma_r}}{12 l^e \beta_r}$$

- Crack path (Topological search, **Bottoni et al. 2015**)

$$[u_{n^e}^e]_{strong} = \frac{(\varepsilon_{FE}^* \phi)(x_0) \int_{\Gamma} \phi(x_0 - x) ds}{\phi(0)}$$

**Dufour et al. 2008**

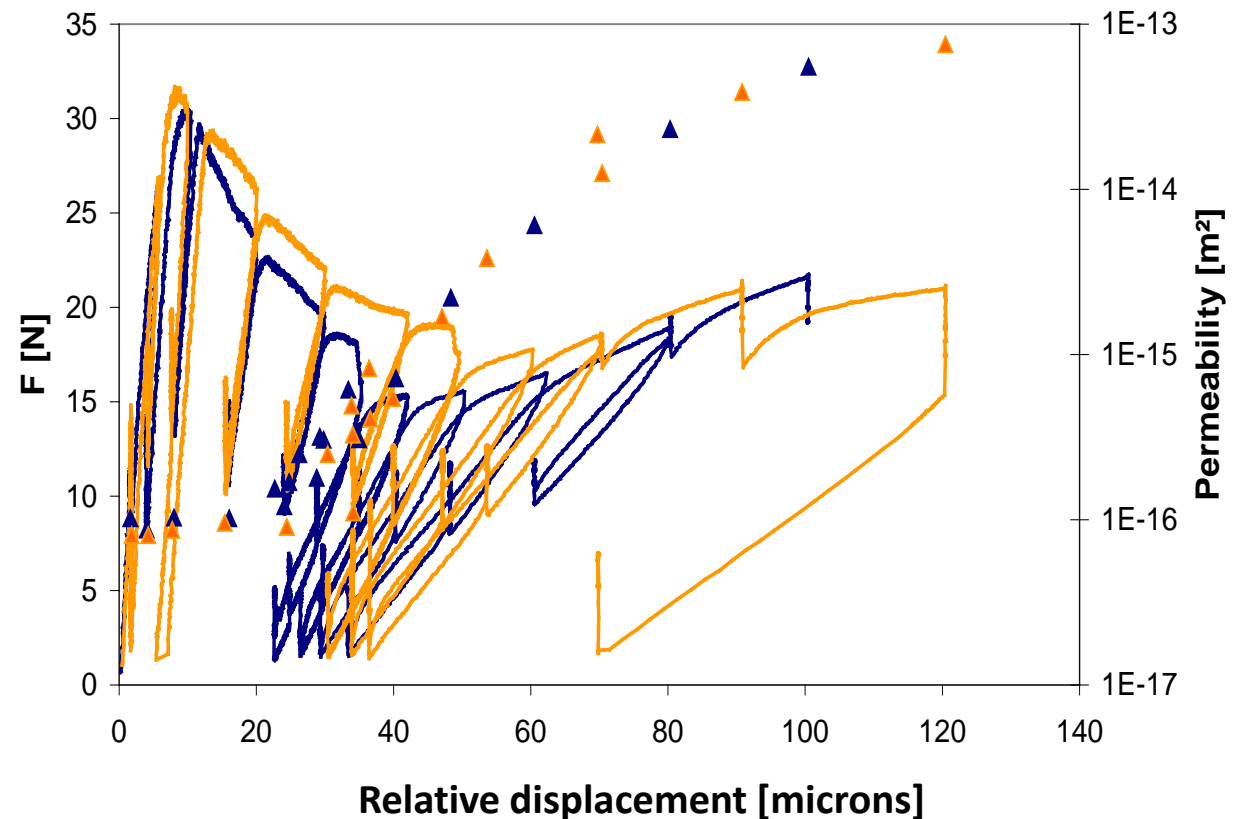
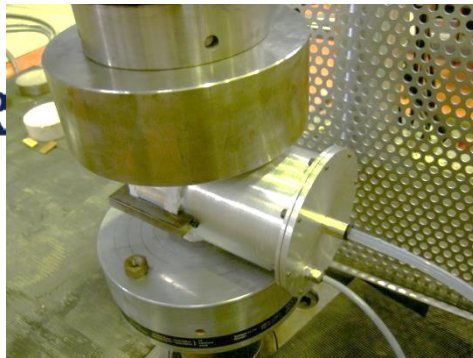
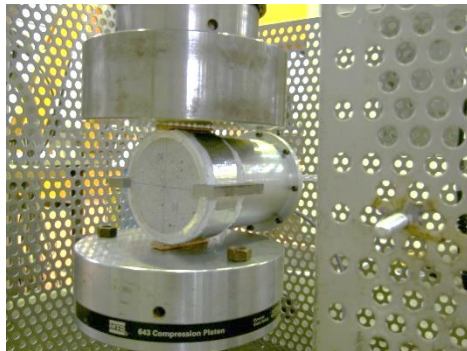
Sum of fluxes  $Q = Q_{bulk} + Q_{crack}$

$$K_m^e = k_D^e I + k_P^e R^T (I - n^e \otimes n^e) R$$

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  - Physical experiment
  - FE simulation
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# ***Physical experiment (Hydro-mechanical behaviour)*** **(Controlled by COD)**

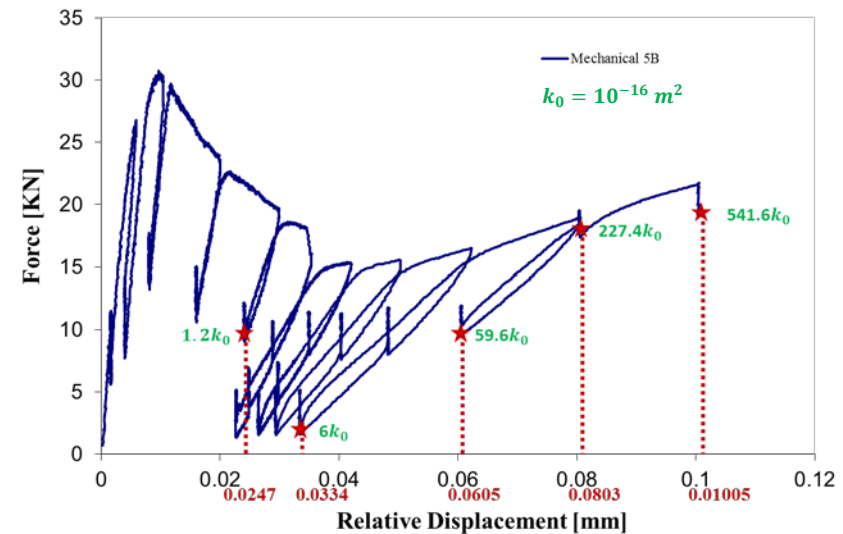
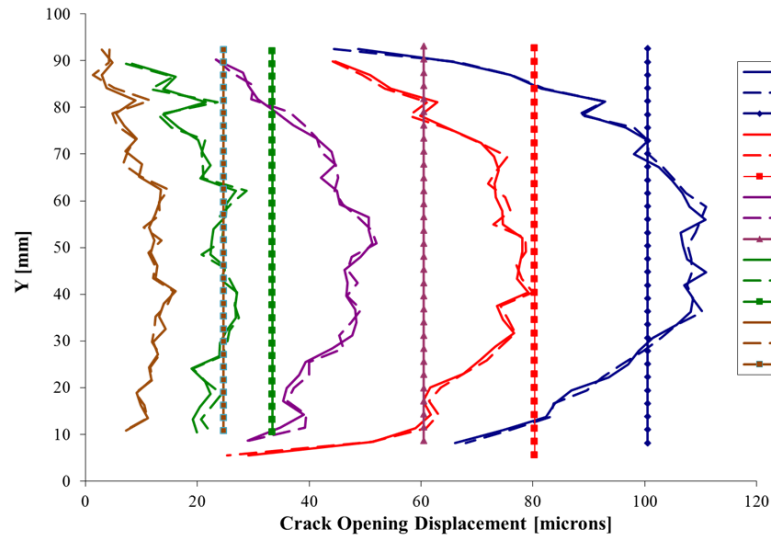


**Gas permeability of mortar under splitting test determined  
when partially unloaded.**

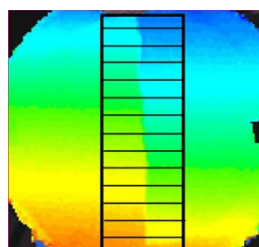
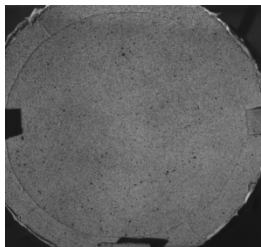
**Dufour 2007 (HDR)**

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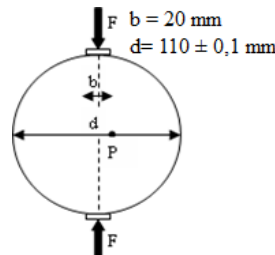
## Physical experiment (Crack opening assessment)



Face  $S_s$



Face  $S_b$



$S_b$  : surface with larger diameter

P: on  $S_b$ , disp. sensor

- 3D effect seen on the cracking patterns due to geometrical effect.

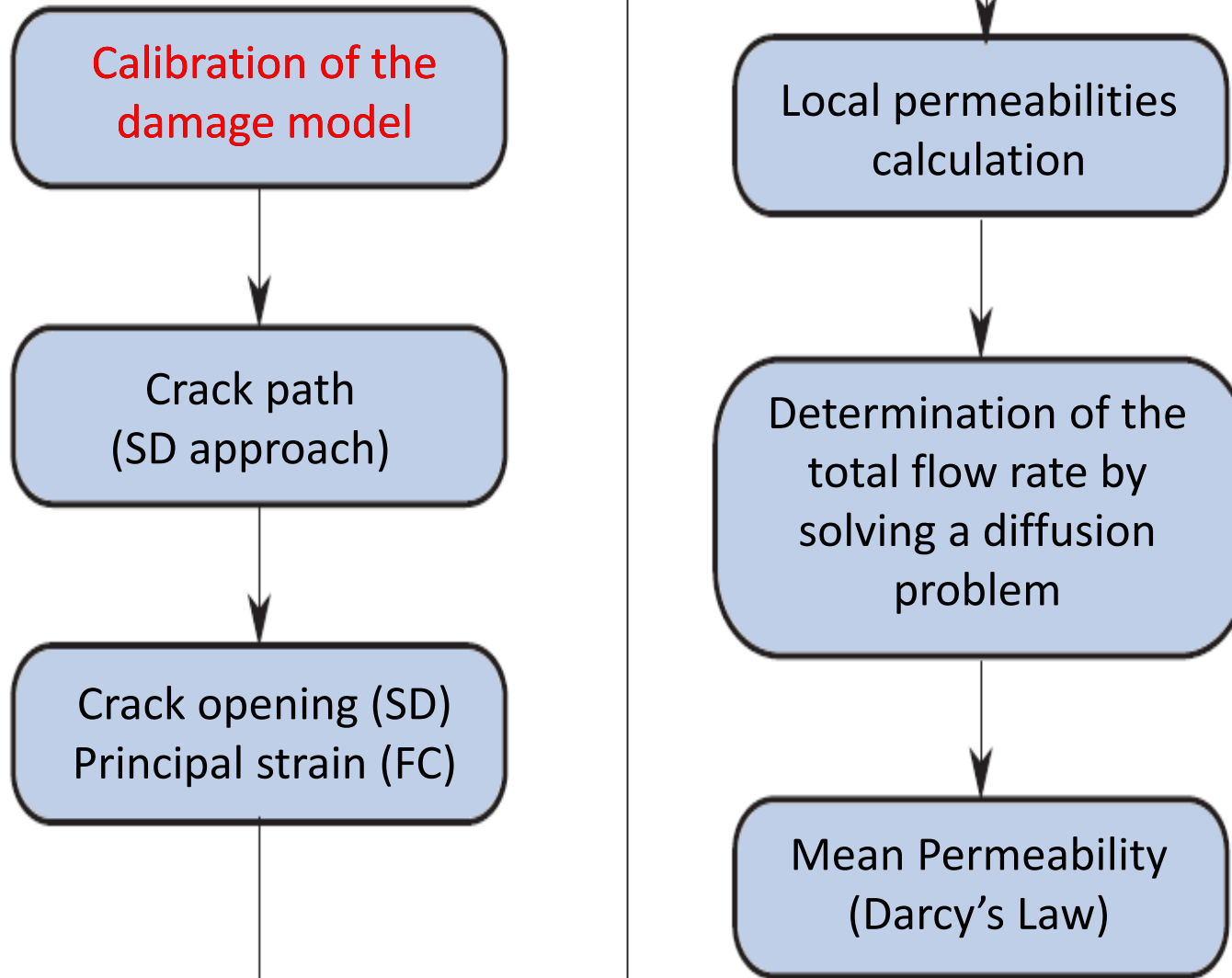


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- ▶ - **Application: Brazilian test**
  - Physical experiment
  - FE simulation**
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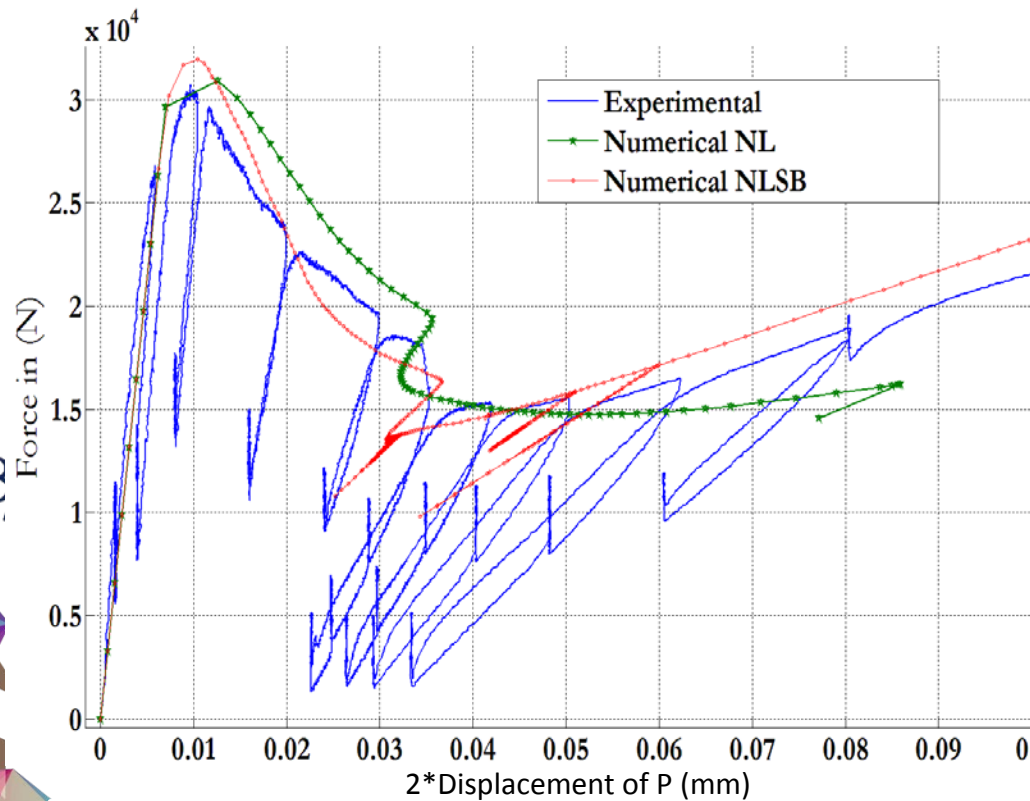
## ***Staggered scheme (weak)***



# Calibration of 3D mechanical model

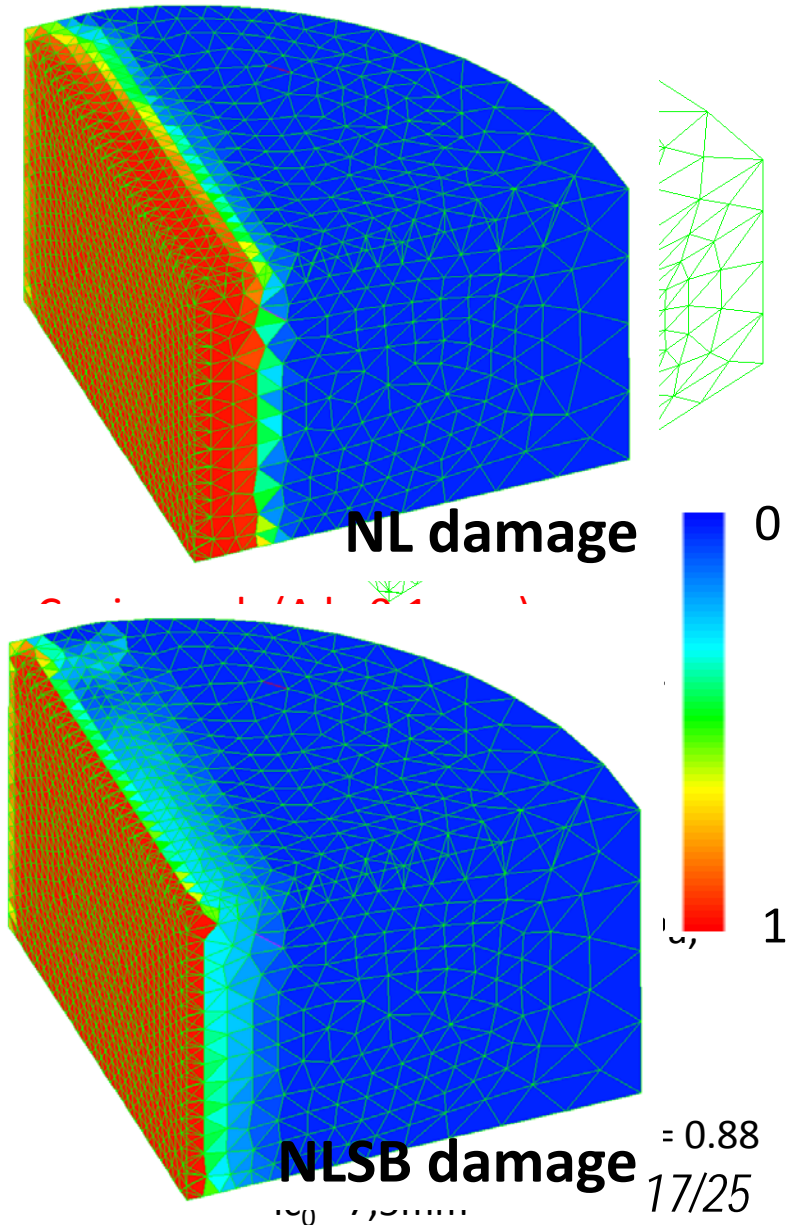
## Arc-length control by maximum strain

(Force-Disp snap back)



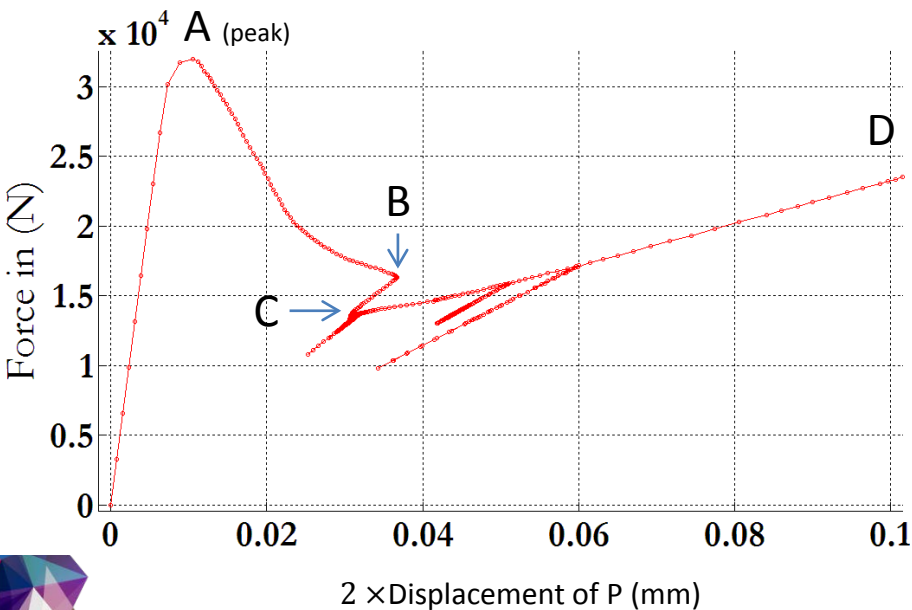
- Better behavior after the splitting phase.
- More realistic damage field.

NLSB

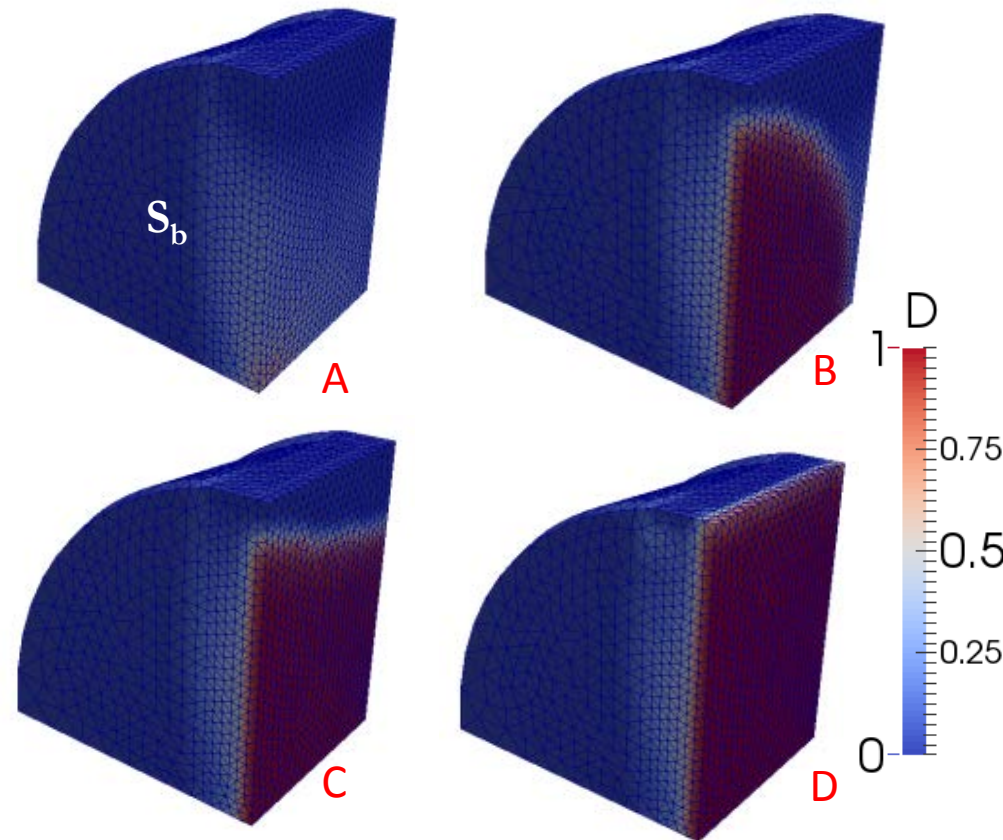


# Damage Profiles

## NLSB model

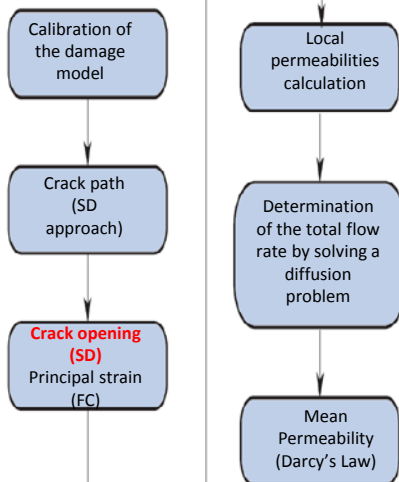
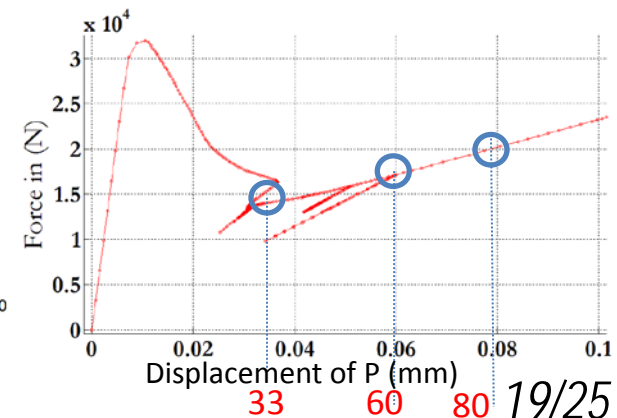
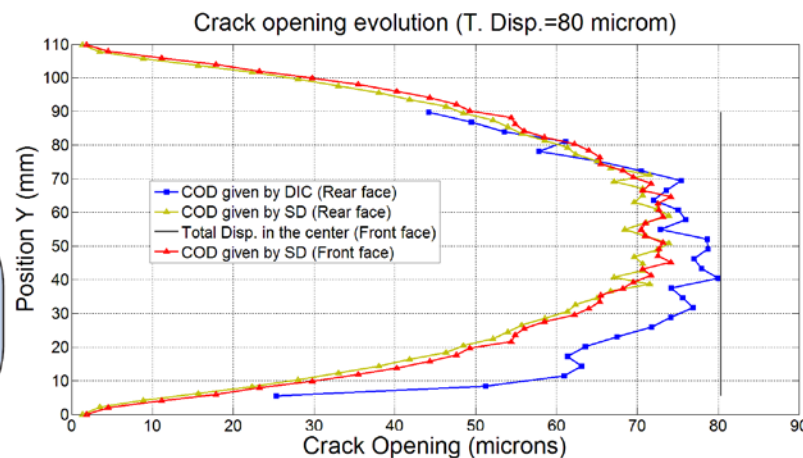
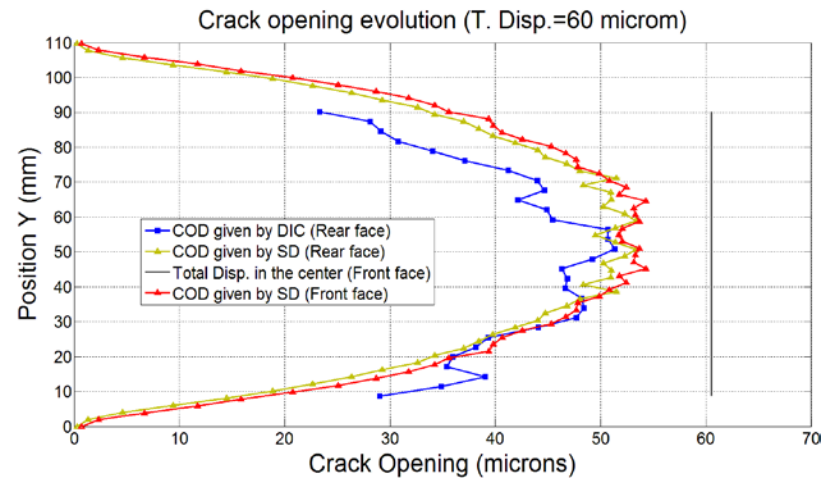
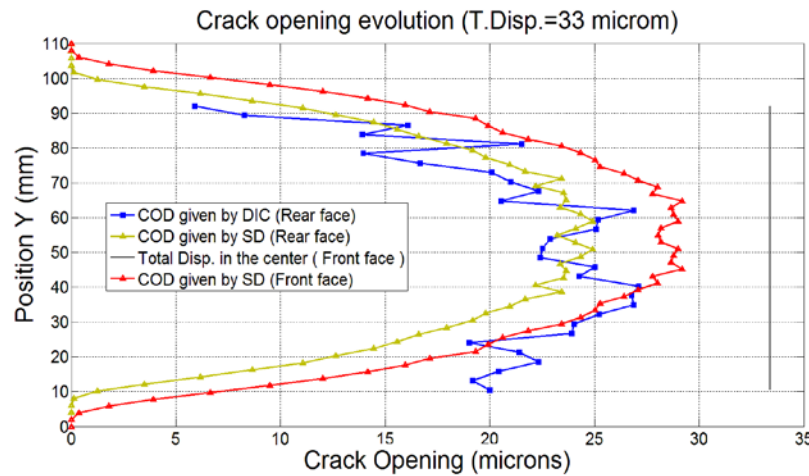


$S_b$  : surface with larger diameter



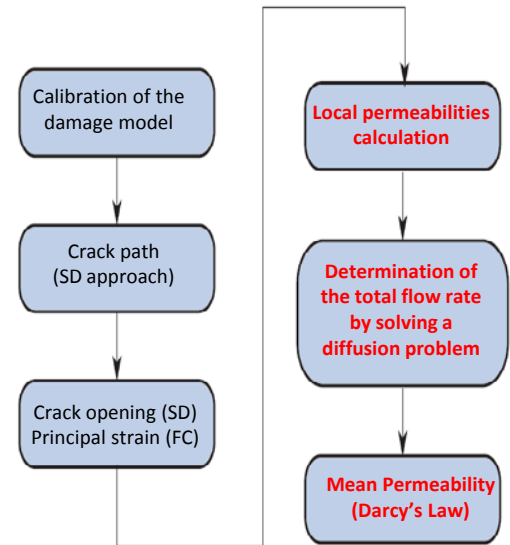
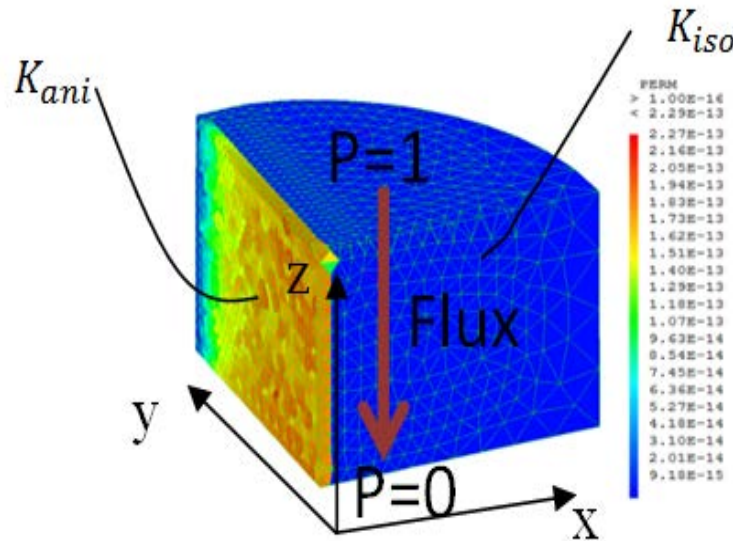
- The 3D simulation highlights the damage (crack) propagation in the longitudinal direction.

# Crack opening assessment






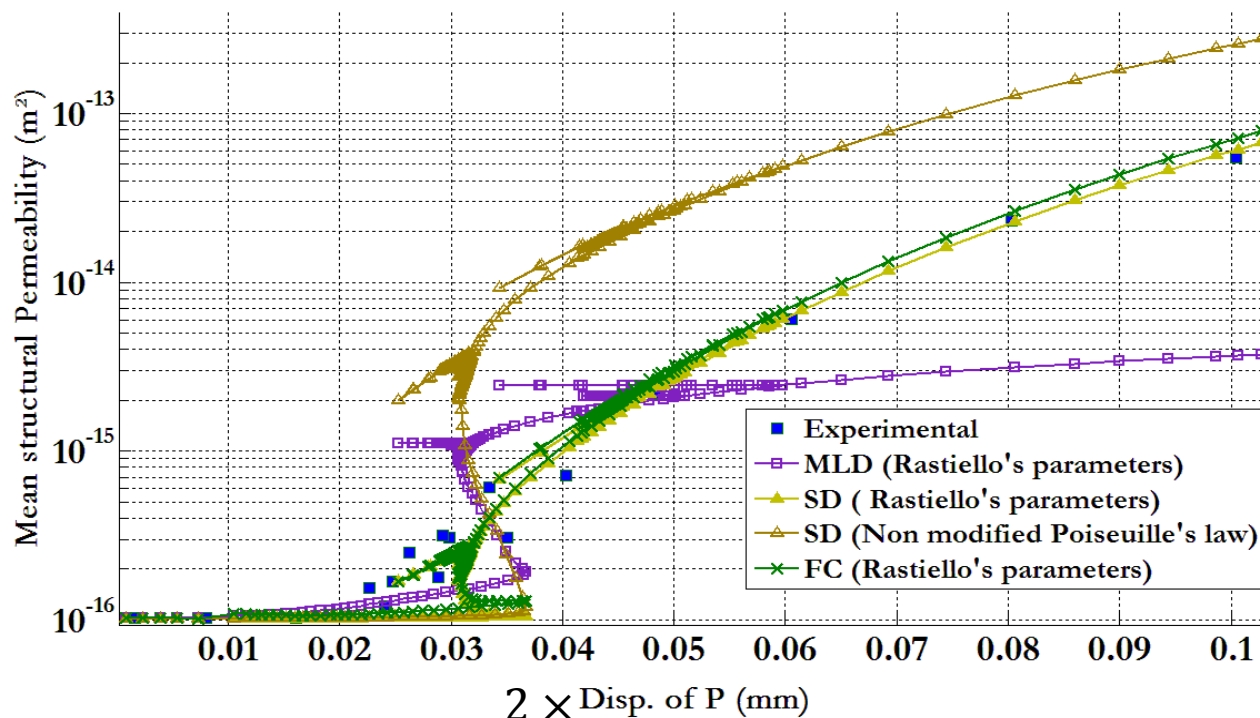
## Local and structural permeabilities calculation



*Solving the diffusion problem by applying a pressure gradient.*


 $Q \text{ is computed and } k_m = \mu \frac{Q}{A} \left( \frac{\Delta P}{\Delta x} \right)^{-1}$

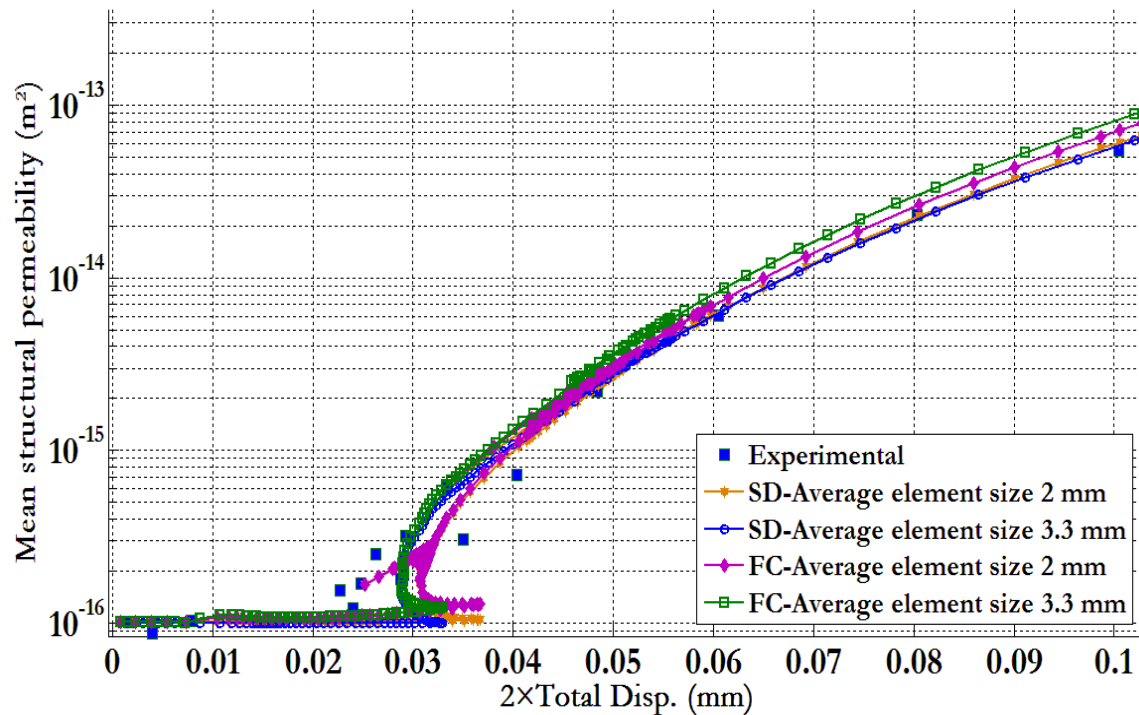
## Coupling between permeability and mechanical state



1. Damage (permeability) stabilizes around 1 and do not evolves when unloading (MLD)
2. Poiseuille's law ( $ksi=1$ ) overestimates the crack permeability
3. Good agreement between the proposed models and experimental data
4. Rastiello's parameters are valid for an OC/mortar up to COD of 0.1 mm

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## Mesh sensitivity



Average element size: 2 mm  
Nb. of nodes: 4511  
Nb. of elements: 20807

Average element size: 3.3 mm  
Nb. of nodes: 1183  
Nb. of elements: 4585

**Mesh independent results obtained with the proposed approaches**



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## ***Conclusions:***

- The NL stress based is proven to be better than the original NL on the Global scale as well as on the local one.
- With (MLD), damage (permeability) stabilizes around 1 and do not evolves when unloading.
- The crack opening assessment is obtained accurately using the Strong Discontinuity approach applied in the post processing phase.
- It is shown that the proposed parameters of Rastiello that intervene in the relation between the correction factor and the crack opening,  $\gamma$  and  $\beta$ , are valid for an ordinary concrete/mortar.
- The coupling using two approaches is validated on the splitting test.
- Mesh independent results are obtained.

## ***Perspectives :***

- Coupling permeability with thermal and/or creep damage.
- Other applications (steel-concrete interface for instance).
- Generalize the approaches to structure elements.
- Consider slip flow (apparent permeability) in the hydraulic models.



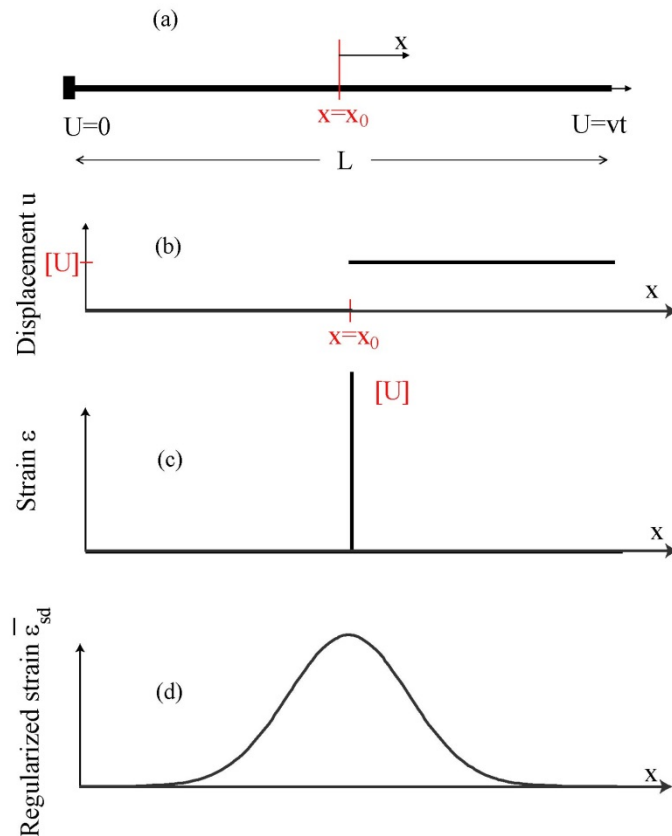
**Thank you for your  
attention**

# Crack location and opening

## Crack path

- Trivial in the actual test (Plane of symmetry parallel to the loading).

## Crack opening along the path



Strong Discontinuity Approach :  
Equality of numerical and analytical profiles at their maximum  $x_0$

$$\bar{\varepsilon}_{sd}(x_0) = \bar{\varepsilon}_{eq}(x_0)$$

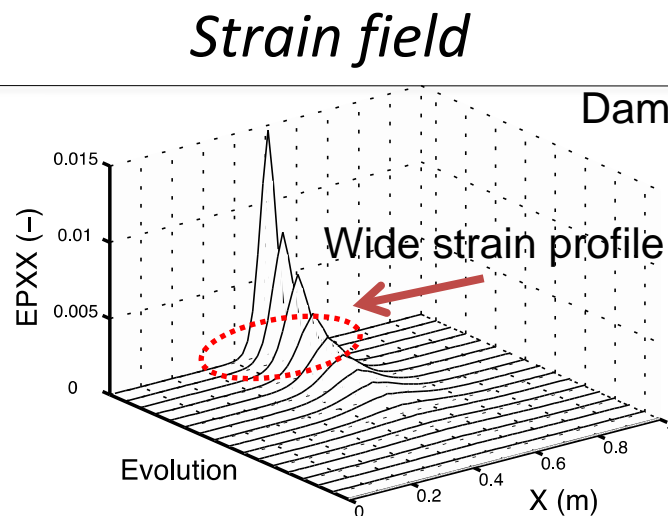
Crack opening

$$[U]_{strong} = \frac{(\varepsilon_{FE} * \phi)(x_0) \int_{\Gamma} \phi(x_0 - x) dx}{\phi(0)}$$

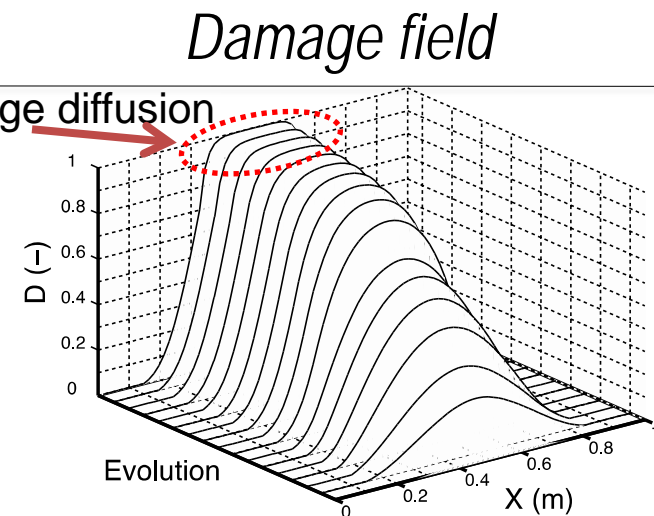
Dufour et al. 2008

# Evolution of the localization at failure

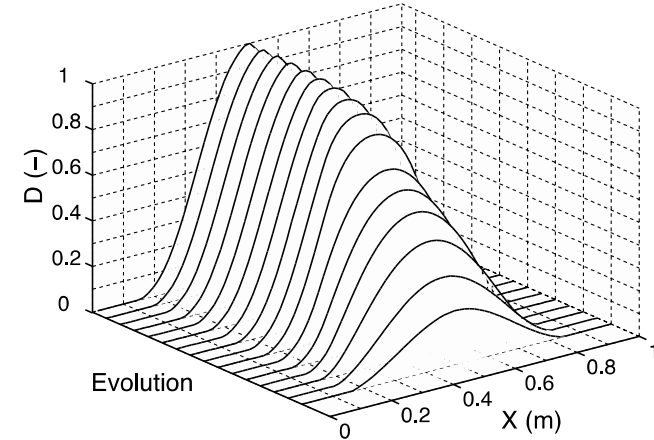
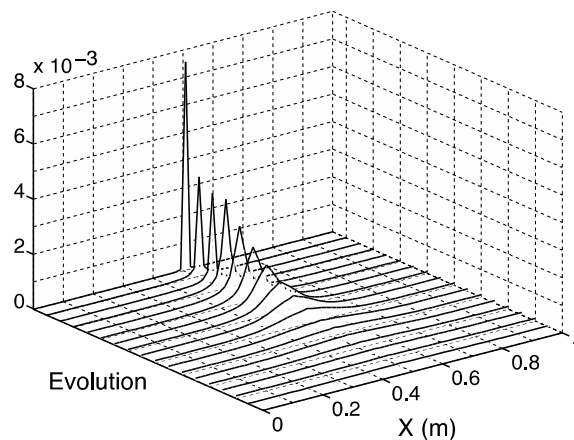
*Original nonlocal model*



Damage diffusion



*Stress based nonlocal model*



Introduction and context	Influence of THM solicitations on the permeability of concrete	Conclusions (EXP.)	Numerical coupling between transport and mechanical properties	Conclusions (NUM.)	Perspectives
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# Applying the SD Method

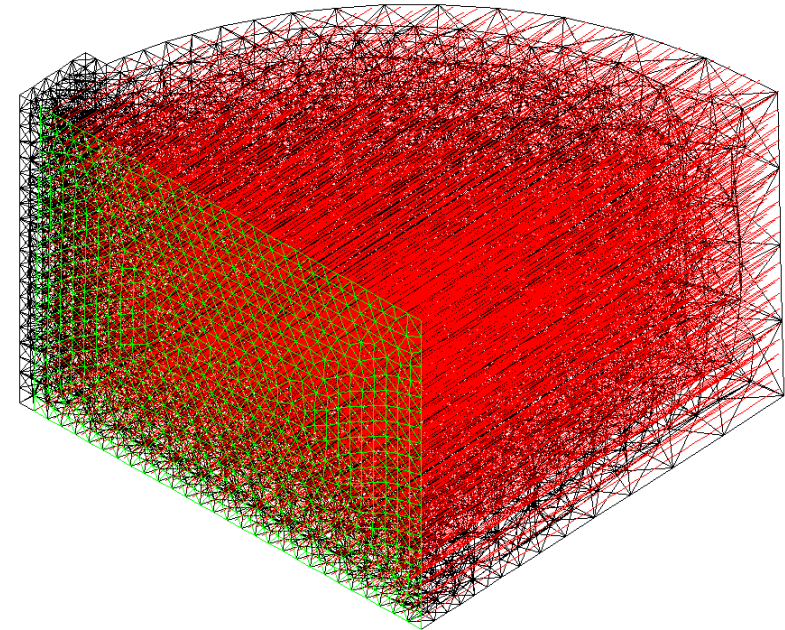
- Crack path is taken as the green crack surface
- Convert the medium to 1D profile along a line orthogonal to the crack surface.
- Projection of strain field on the 1D profile.

$\mathbb{3}^R$

$$\varepsilon_N = \vec{N} \cdot \varepsilon \cdot \vec{N}$$

- Equality of numerical and analytical profiles at their maximum  $x_0$

$$\bar{\varepsilon}_{sd}(x_0) = \bar{\varepsilon}_{eq}(x_0)$$





# Coupling by means of two approaches

Pijaudier-Cabot et al, JEM (2009)



Damage (D)  $\xrightarrow{K(D)}$  Permeability (K)

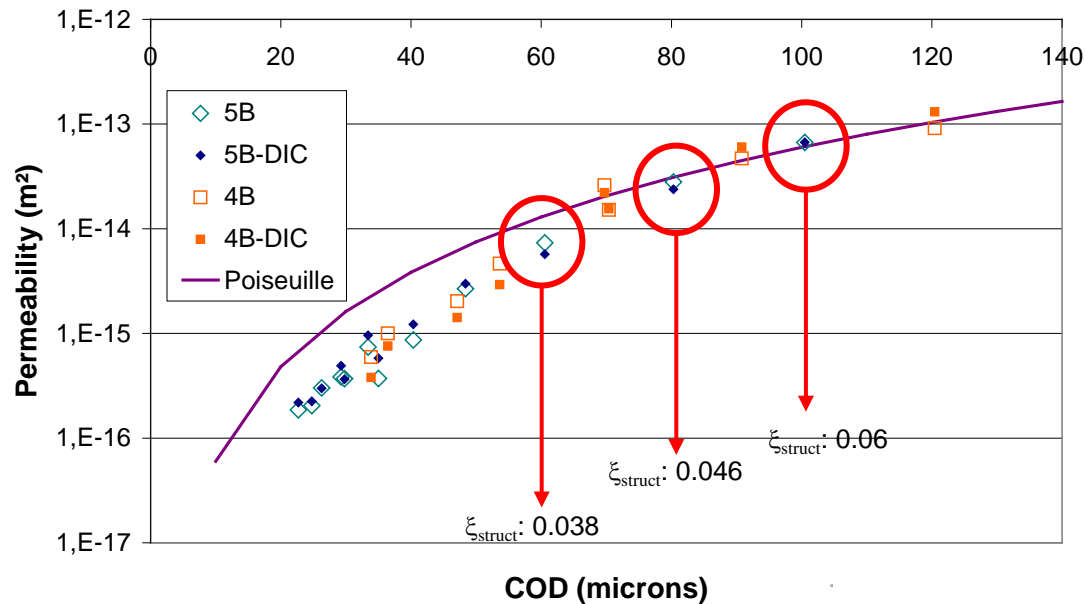
$\log K_{opening} = D \log K_p + (1-D) \log K_0$   
 calculation Poiseuille's law ( $K_p$ )

$$[U]_{strong} = \frac{(\varepsilon_{FE} * \phi)(x_0) \int_{\Gamma} \phi(x_0 - x) ds}{\phi(0)} \quad \text{Dufour et al. 2008} \quad K_P = \frac{[U]^2}{12\alpha}$$

$$[U]_{weak} = \frac{\iint_{\Gamma} (\varepsilon_{FE} * \phi)(s) ds}{\int_{\Gamma} \phi(x - x_0) ds} \quad \text{Dufour et al. 2012}$$



## Correction factor for Poiseuille's law



Poiseuille's law (permeability of a crack)

Structural or mean permeability

$$K_m = K_P \frac{[u]_m \cdot \varphi}{S_{struct}} = \frac{\xi_{struct}}{S_{struct}} \frac{[u]_m^3 \cdot \varphi}{12}$$

$$\xi_{struct} = \frac{1}{\beta [u]^\gamma} \quad \left\{ \begin{array}{l} \beta = 5,625 \times 10^{-5} \\ \gamma = -1,19 \end{array} \right.$$

